Exhibit I. Tamanend Business Park East Site Potable Water Infrastructure Upgrade Letter & Map





Tamanend Business Park East Site Potable Water Infrastructure Upgrade Letter & Map

CSRS IMAGINE SHAPE DELIVER CSRS, INC. 6767 Perkins Road, Suite 200 Baton Rouge, Louisiana 70808 Phone: (225) 769-0546 Fax: (225) 767-0060

June 14, 2016

Mr. Michael Tomlinson
St. Tammany Economic Development Foundation
21489 Koop Drive, Suite 7
Mandeville, LA 70471
Re. Tamanend Business Park East Site Potable Water System Cost Estimate CSRS Job No. 214094

Dear Mr. Tomlinson:

According to correspondence with local utility officials, the Tamanend Business Park East site located in St. Tammany Parish, Louisiana currently does not have access to an existing potable/process water line to service the site. A plan to improve and upgrade the water infrastructure is in place.

A sewer and water master plan has been developed by Richard C. Lambert Consultants. In addition, a sewer and water design plan has been provided for the Tamanend Business Park East site and infrastructure upgrades are currently being constructed.

Regarding the potable water capacity, the system has been designed and partially constructed, but is currently not in active use. A Tamanend Water System Storage, Production & Pressure Recommendations report has been provided by Principal Engineering, Inc. The report shows that the proposed water system will meet LED requirements.

Thank you for the opportunity to assist you in this project. Should you have any questions or require additional information, feel free to contact me.

Sincerely, CSRS, Inc. Taylor M. Gravois, PE, PLS Taylor M. Gravois, PE, PLS

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- 1. No attempt has been made by CSRS, Inc. to verify site boundary, title, actual legal ownership, deed restrictions, servitudes, easements, or other burdens on the property, other than that furnished by the client or his representative.
- Transportation data from 2013 TIGER datasets via U.S. Census Bureau at ftp://ftp/.census.gov/geo/tiger/TIGER2013.
 Utility information from visual inspection and/or the individual utility operators. Exact field location has not been determined by survey. The lines shown are an approximate representation only and may have been offset for depiction purposes.
 2015 aerial imagery from USDA-APFO National Agricultural Inventory Project (NAIP) and may not reflect current ground conditions.



Potable Water Tamanend Business Park East Site St. Tammany Parish, LA

St. Tammany EDF





Date:	6/13/2016
Project Number:	214094
Drawn By:	ТМК
Checked By:	JAY



Water Utility Provider Survey (page 1 of 2)

214094 Project ID



Tamanend Business Park East Site Potable Water Infrastructure Upgrade Letter & Map

Site Map 1				Site Map 2		
Date:	06/01/2016			Contac	t Information	
Provider Name:	Tammany Utilities			Name:	Tim Brown	
Address:	21454 Koop Drive			Phone:	(985) 898-2535	
City:	Mandeville			Email:	tbrown@stpgov.org	
State:	LA	Zip Code:	70471	Title:	Utility Manager	
Is potable or p currently avail	rocess water able at this site?	Please pr process v	ovide the distance in water distribution lin	n feet to the r e to service t	nearest potable or this site.	What is the size (inches in diameter) of the nearest line?
⊖ Yes	No	100				16
What are the pr	essures of the water	line at or ne	arest to this site?	Static 0	Re	sidual 0
Source of potat	ble or process water (lake, well, of	ther source) gr	oundwater		
What is the total p	ootable/process capacit	y of the existin	ng water system in million	is of gallons pe	r day (MGD)	0
What is the cur 0	rent average daily us	e of the exist	ting water system in m	nillions of gallo	ons per day (MGD)	0
What is the pea	k demand on the exi	sting water s	system in millions of g	allons perday	r (MGD) 0	
What is the exc	ess capacity of the e	xisting water	system in millions of	gallons per da	ay (MGD) 0	
Capacity of clos	sest elevated potable	water stora	ge tank (gallons)		Ę	500,000
Distance to closest elevated potable water storage tank in miles 0.04 Distance to appropriate booster station in miles 0						
Is or will there be adequate pressure and flow at site to combat fires?						
Is a plan underway to improve services at or near this site within the next year? If so, please provide anticipated upgrades, location, and time for implementation.						
Facility is still under construction. The pump and well tests have not been completed. As-built plans have not yet been provided.						





Tamanend Business Park East Site Potable Water Infrastructure Upgrade Letter & Map

TAMANEND WATER SYSTEM STORAGE, PRODUCTION, & PRESSURE RECOMMENDATIONS

PREPARED FOR:



CALDWELL TANKS, INC.

JULY 2014



PREPARED BY: PRINCIPAL ENGINEERING, INC.



1011 N. Causeway Blvd., Suite 19 Mandeville, LA 70471 Ph. (985)624-5001 – Fax (985)624-5303

PURPOSE

This document summarizes a limited study conducted by Principal Engineering, Inc., on the proposed water storage, production, and distribution system for the Weyerhauser development, Tamanend. The study purpose is to provide the developer with data to validate the selected storage capacity/height and distribution network pipe diameters or make revisions to same in the interest of better economic outcome. No deficiency in the current design is implied, and the parcels as located on the master plan prepared by Reich Associates are constructible with respect to water supply, but it is possible that certain modifications would lessen the cost of building construction and increase usable land area. Additionally, no water supply design well capacity has been selected; this document presents a rationale and recommendation for water production capacity.

EXECUTIVE SUMMARY

Within the framework of National Fire Protection Association (NFPA) regulations adopted by the State of Louisiana, and the physical behavior of a water system; storage capacity, production capacity, water pressure, pipe diameters, and building type/use are interconnected. Modification to one element's characteristics may place constraints on another element. The potential variations are endless. As such, a set of design assumptions have been selected, and the recommendations made based on those assumptions. Matrices of broader results are included later in this document for the interested reader (the matrices are limited in scope for reasonableness).

- <u>Tank Capacity.</u> The assumed water storage tank capacity of 500,000 gallons is validated. The controlling case is the demand created by fire suppression sprinklers in a warehouse (1,860 gpm for 90 minutes) during a period of high domestic usage. No auxiliary water storage is required for the assumed 60,000-100,000 SF warehouse (4,000 SF sprinkler area plus hose stream, ordinary hazard), when the proposed 500,000 gallon tank is provided, and the recommended well capacity is installed. Demand can vary greatly based on materials stored, method of storage, rack height, sprinkler type, etc.; used here was a conservative configuration.
- 2. <u>Well Production Capacity</u>.
 - a. <u>Primary Capacity</u> is recommended to be 1,000 gpm, although in no case should it be less than 750 gpm. The two controlling cases for well capacity are 1) domestic demand when the storage tank is out of service; and 2) fire demand triggered at the end of a maximum domestic use day (for a storage capacity of 500,000 gallons). Peak domestic (non-fire) hourly demand at full build-out is estimated at 1,235 gpm in summer, and 987 gpm in winter. While tank maintenance is performed, 1000 gpm production capacity will maintain system pressure during the

winter months at all times of day. It is not recommended to size the primary well down, and rely on the auxiliary and primary sources in combination for this purpose, as redundancy is lost. *If possible, it is recommended that two 500 or 600 gpm wells be installed as the primary source for maximum redundancy, vs. a single well of 1000 gpm capacity*. Depending on the ultimate build out of Phase 1, well construction could be phased.

- b. <u>Auxiliary Capacity</u> is recommended to be 500 gpm. This value is an estimate of the most water the existing Coroner's well can produce with a pump and controls upgrade. For higher capacity than 500 gpm, redrilling would be required, negating the benefit of using that existing facility. The capacity will be adequate to provide redundancy during Phase I development; however, as the demand increases, and particularly if a single 1000 gpm or 750 gpm well is installed as the primary source, it will be inadequate during days of peak summer domestic demand (when in redundant use). The 500 gpm auxiliary source can likely supply winter domestic demand at build-out, but in no case can fire demand be met.
- 3. <u>Pressure</u>. It is recommended to maintain the proposed HWL height of 140 feet, and to increase the water main diameters serving the office/warehouse parcels from 10" to 16". Installation of a fire pump in the major buildings can be avoided (for most usage cases) with this diameter increase. Raising the tank elevation sufficiently to provide adequate sprinkler head pressure at the aforementioned warehouse fire flow without a pump, and using the 10" pipe diameter would require tank height approaching 200 feet; this is considered impractical. With the tank level depleted to 120 feet during a day's use, pressure and flow have been validated as adequate at the most hydraulically distant fire hydrant from the tower, at the proposed pipe diameters.
- 4. Economic Interpretation of #1 thru #3.
 - a. <u>Storage</u>: Employing the recommended 500,000 gallon storage tank capacity and 1000 gpm production capacity will eliminate the need for fire water ground storage tanks located adjacent to each of the office and warehouse buildings. For reference, a single 50,000 gallon steel water storage tank would cost over \$100,000. It is conceivable that each of the 12 warehouse/office facilities would require such a tank, in addition to any single large commercial area in the Town Center.
 - <u>Production</u>: As primary production, two wells of smaller capacity are desired vs. one well of higher capacity. A single 500 gpm well can be budgeted at \$450,000; and a single 1000 gpm well can be budgeted at

\$700,000 (bare costs, well and pump only). The higher total construction cost of two smaller wells (\$900K vs. \$700K) negates risk of required future additional upgrade to the auxiliary source as build-out occurs. It is possible that during DHH review, this two well primary supply approach will be mandated (or other solution that delivers complete redundancy).

- c. <u>Pressure</u>: Increasing the water main distribution diameter will cost an estimated additional \$20 per linear foot (nominal additional cost for Phase 1, and \$114,000 additional cost during future build out for 5700 LF of distribution mains to the warehouses). However, this will eliminate the need for a fire pump in most potential facility uses. The estimated per parcel construction savings (assume 12 warehouses/offices plus town center and campus) is \$25,000 in upfront cost, plus the O&M necessary to ensure that equipment's reliable operation.
- d. <u>No Action Required</u>: As previously stated, no deficiency in the distribution design is implied. Should the recommended 1000 gpm production capacity and pipe diameter increase not be implemented, the development will not be technically hindered; but building construction will become more expensive as described above.

LIMITATIONS

Water demands have been estimated using the rendered plan produced by Reich Associates, and not on definitive information. It is possible that changes to the Tamanend layout or land usage could render the information contained herein invalid. Additionally, a wide variety of fire suppression sprinkler types, configurations, design methods, and building characteristics can produce required flows and pressures above or below what is presented herein. Principal Engineering has made an effort to present a reasonably conservative envelope, but cannot guarantee that if the recommendations are followed, that certain building owners will not require fire pumps. Lastly, the cost figures are for comparison purposes only, and are based on past experience, not proposed pricing by Contractors. As such, the developer should make independent investigation to verify accuracy.

TABULATED RESULTS OF ANALYSIS

Selected tables of results are presented below.

<u>Table 1, Domestic Use Values</u>: This table presents water use that can be expected upon full build-out of the property, according to the Reich Associates plan. Hourly max flow during winter months has been assumed as (base flow) x 0.8 x 2.5.

	Peaking Factor	Flow (gpd)	Flow (gpcd)	Flow (gpm)
Base	N/A	711,060	94.5	493.8
Winter	0.8	568,848	75.6	-
Summer	1.3	924,378	122.8	-
Daily Max	1.65	1,173,249	155.9	-
Hourly Max	2.5	-	236.2	1,235

<u>Table 2,Volume Deficit for Fire Demand Duration of 90 min.</u>: This table presents the deficit in available water volume for various well production capacities and various building hazard classifications. Fire sprinkler demands were estimated for warehouse/office buildings of varying hazard classifications.</u>

Fire Demand varies & Domestic Demand = 1,235 gpm								
	Water Well Capacity (gpm)							
Hazard								
Class	500	750	1000	1500	2000	2500	3000	
Ordinary 1	124,650	102,150	79,650	34,650	N/A	N/A	N/A	
Ordinary 2	142,650	120,150	97,650	52,650	7,650	N/A	N/A	
Extra								
Hazard 1	197,550	175,050	152,550	107,550	62,550	17,550	N/A	
Extra								
Hazard 2	233,550	211,050	188,550	143,550	98,550	53,550	8,550	

2 - Volume Deficit (gal) for Fire Demand of 90 minutes Fire Demand varies & Domestic Demand = 1.235 gpm

<u>Table 3, Volume Deficit for Peak Domestic Flow</u>: This table presents the deficit in available water volume for various well production capacities during a peak domestic use day. The maximum expected hourly flow is assumed to be maintained until the maximum expected daily production for a peak day is reached (calculated to be 16 hr.).

 (1,235 gpin 101 10 m.)								
Water Well Capacity (gpm)								
500 750 1000 1500 2000 2500 3000								
705,600	465,600	225,600	0	0	0	0		

3 - Volume Deficit (gal) for Peak Domestic Flow (1,235 gpm for 16 hr.)

<u>Table 4, Volume Deficit for Peak Domestic + Fire Demand:</u> This table sums the values of volume deficit in Tables 2 and 3. This models a fire sprinkler demand at the design office/warehouse building on a peak domestic usage day.

	Water Well Capacity (gpm)							
Hazard								
Class	500	750	1000	1500	2000	2500	3000	
Ordinary 1	830,250	567,750	305,250	34,650	0	0	0	
Ordinary 2	848,250	585,750	323,250	52,650	7,650	0	0	
Extra								
Hazard 1	903,150	640,650	378,150	107,550	62,550	17,550	0	
Extra								
Hazard 2	939,150	676,650	414,150	143,550	98,550	53,550	8,550	

4 - Volume Deficit (gal)	for Peak Domestic Flow of	f 16 hours + Fire Demand

<u>Tables 5A – 5C, Design Parameters for Various Pipe Diameter Configurations:</u> This series of tables estimates the instantaneous flow of water to the most hydraulically distant office/warehouse parcel, given varying pipe diameters and tank water levels. The highlighted values are the flow that can be expected for the given configuration at a point 40 ft above ground elevation (assumed roofline), at a residual pressure of 30 psi. Pressure

loss of 10 psi has been assumed in the fire suppression riser piping. The tank level shown represents a tank HWL height 20 feet higher (120 in the table is a 140' tank), to account for water volume depletion. Topography has been assumed as flat. Criteria is the 1,860 gpm fire flow mentioned above.

•		-	
Tank Level (ft)	Pipe Section	Velocity (ft/s)	Flow (gpm)
120	16" Main Pipe	1.87	1,629
120	10" Spur Pipe	4.80	1,174
140	16" Main Pipe	2.51	2,179
	10" Spur Pipe	6.42	1,570
170	16" Main Pipe	3.26	2,834
	10" Spur Pipe	8.35	2,042
180	16" Main Pipe	3.48	3,024
	10" Spur Pipe	8.91	2,179

5A - Design Parameters for 16" Main Pipe & 10" North Spur Pipe

5B - Design Parameters for 16" Main Pipe & 12" North Spur Pipe

Tank Level (ft)	Pipe Section	Velocity (ft/s)	Flow (gpm)
120	16" Main Pipe	2.57	2,234
120	12" Spur Pipe	4.57	1,610
140	16" Main Pipe	3.44	2,987
	12" Spur Pipe	6.11	2,153
170	16" Main Pipe	4.47	3,886
	12" Spur Pipe	7.95	2,801
180	16" Main Pipe	4.77	4,145
	12" Spur Pipe	8.48	2,988

5C - Design Parameters for 16" Main Pipe & 16" North Spur Pipe

Tank Level (ft)	Pipe Section	Velocity (ft/s)	Flow (gpm)
120	16" Main Pipe	5.88	5,112
120	16" Spur Pipe	5.88	5,112
140	16" Main Pipe	7.88	6,850
	16" Spur Pipe	7.88	6,850
170	16" Main Pipe	10.23	8,893
170	16" Spur Pipe	10.23	8,893
180	16" Main Pipe	10.92	9,493
	16" Spur Pipe	10.92	9,493

DESIGN PARAMETER CONFIRMATION

- 1. The elevated storage tank will be designed to a capacity of 500,000 gallons, with a high water level of 140 feet above foundation elevation.
- A single well of 500 gpm will be designed at the tank site. A future 500 gpm well shown on the drawings will be required as Tamanend develops and water demand increases. An upgrade to the Coroner's well will be designed to flow 500 gpm.

Approved:_____

Date:_____

Signature

Printed Name:_____