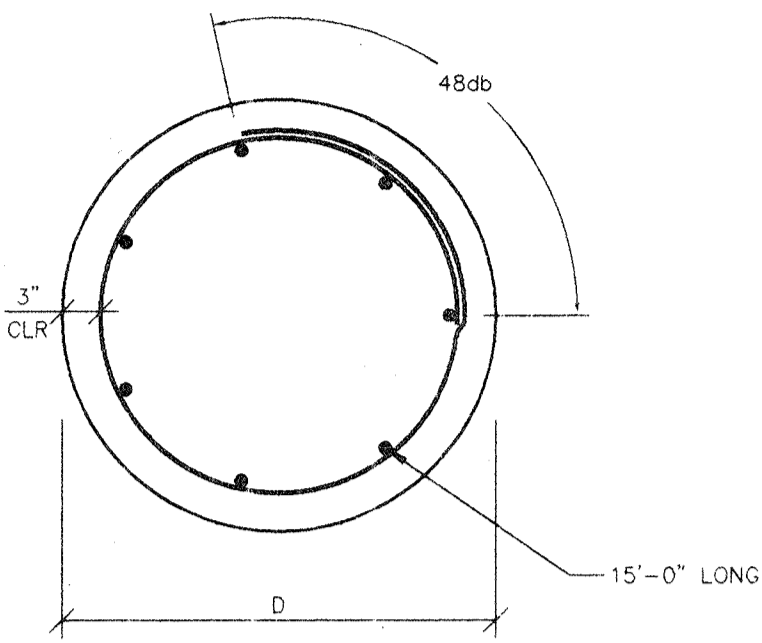
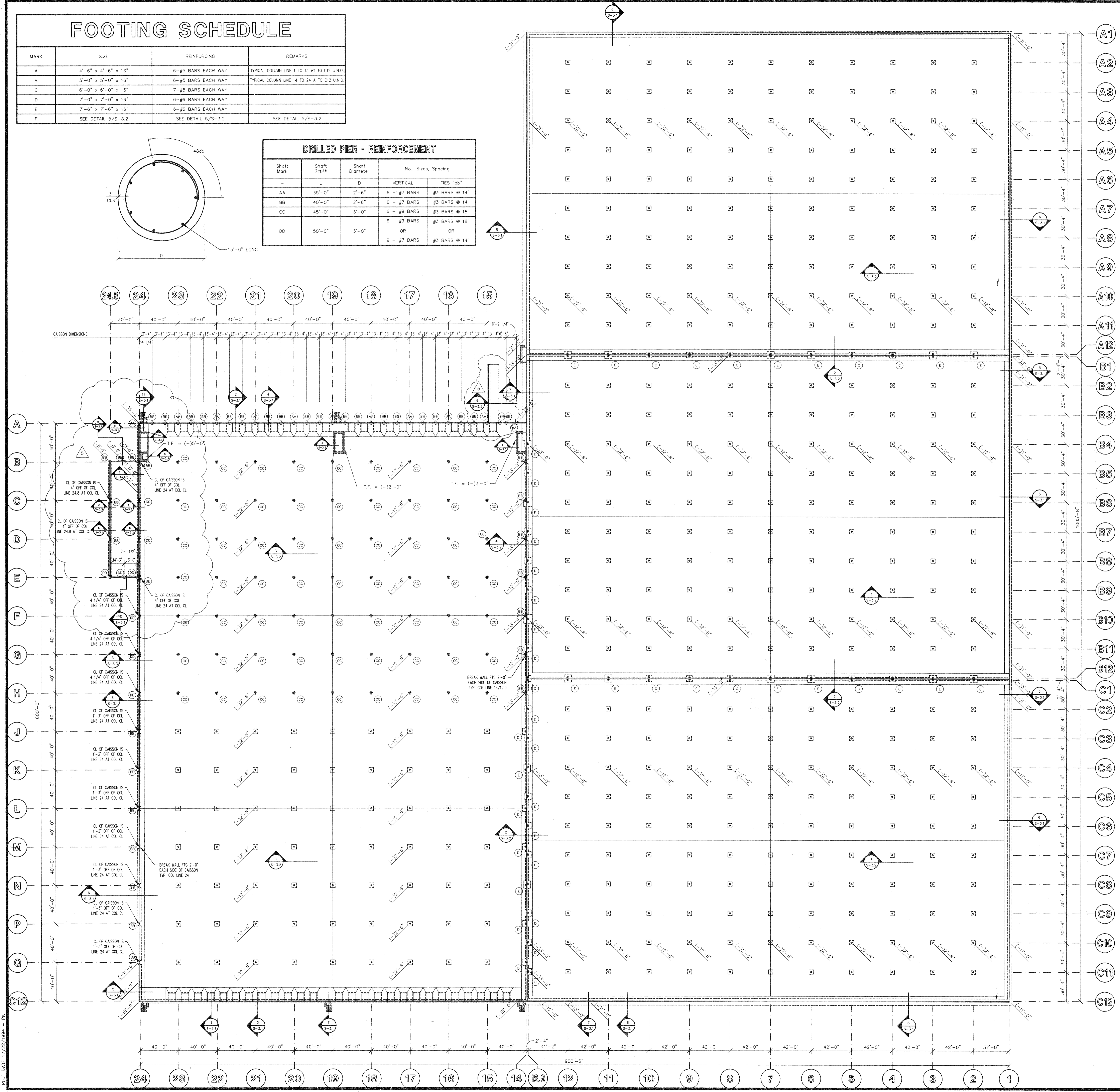


# FOOTING SCHEDULE

MARK	SIZE	REINFORCING	REMARKS
A	4'-6" x 4'-6" x 16"	6-#5 BARS EACH WAY	TYPICAL COLUMN LINE 1 TO 13 AT TO C12 U.N.O.
B	5'-0" x 5'-0" x 16"	6-#5 BARS EACH WAY	TYPICAL COLUMN LINE 14 TO 24 AT TO C12 U.N.O.
C	6'-0" x 6'-0" x 16"	7-#5 BARS EACH WAY	
D	7'-0" x 7'-0" x 16"	6-#6 BARS EACH WAY	
E	7'-6" x 7'-6" x 16"	6-#6 BARS EACH WAY	
F	SEE DETAIL 5/5-3.2	SEE DETAIL 5/5-3.2	SEE DETAIL 5/5-3.2



Shaft Mark	Shaft Depth	Shaft Diameter	No. Sizes, Spacing	
			VERTICAL	TIES "db"
AA	35'-0"	2'-6"	6 - #7 BARS	#3 BARS @ 14"
BB	40'-0"	2'-6"	6 - #7 BARS	#3 BARS @ 14"
CC	45'-0"	3'-0"	6 - #9 BARS	#3 BARS @ 16"
DD	50'-0"	3'-0"	6 - #9 BARS OR 9 - #7 BARS	#3 BARS @ 16" OR #3 BARS @ 14"



## GENERAL NOTES

- DESIGN DATA:
- a) Applicable Code: Standard Building Code 1991 Edition
  - b) Design Loads:
    - Dead Load:
      - Roofing and insulation: 3 lbs./square foot
      - Roof Deck: 2 lbs./square foot
      - Joists: 2 lbs./square foot
      - Girders: 2 lbs./square foot
      - Sprinklers: 2 lbs./square foot
      - Mechanical/Electrical: 3 lbs./square foot
      - Process Piping (as noted): 0 lbs./square foot
    - Live Load:
      - Roof:
        - 0-200 square feet: 20 lbs./square foot
        - 201-600 square feet: 16 lbs./square foot
        - 601 square feet: 12 lbs./square foot
      - Snow Load: 5 lbs./square foot
      - Wind Load: 80 MPH - Frame; 80 MPH - Components
      - Earth Quake: Zone 1
    - c) Soil Bearing Allowable: 2.1 Kips/square foot (isolated footings); 1.6 Kips/square foot (wall footings)

- CONCRETE:
- a) Concrete at interior slabs on grade and tilt-up panels shall have a minimum compressive strength of 4,000 psi at 28 days. All other concrete shall have a minimum compressive strength of 3,000 psi at 28 days.
  - b) Design and construction shall conform to the "Building Code Requirements for Reinforced Concrete Construction" and details shall conform to the "Manual of Standard Practice for Detailing Reinforced Concrete Structures" both the latest editions by the American Concrete Institute unless otherwise shown or specified.

- REINFORCING STEEL:
- a) All reinforcing steel to be Grade 60.
  - b) All reinforcing shall conform with CRSI Standards.
  - c) Reinforcing steel shall lap 48 bar diameters at all splices in masonry, class "B" tension lap splices in concrete unless otherwise shown on the drawings.
  - d) All bars in masonry walls shall return 48 bar diameters at corners and junctures of masonry walls.
  - e) All bars in concrete walls shall return #/ class "B" tension splice at corners and junctures of concrete walls.
  - f) Unless otherwise shown the clear cover for reinforcing bars shall be:
    - 1) Walls exposed to ground or weather: 2"
    - 2) Walls and footings placed on the ground without forms: 3"
    - 3) Beams, columns and piers: 2"

- STRUCTURAL STEEL:
- a) All structural steel, plates and bars to be ASTM A36 unless noted otherwise, Bolts A325.
  - b) All structural steel tubing to be ASTM A500, grade B.
  - c) Light Gauge Cold-Formed Channel Girts: Fy 50 KSI, Fd 30 KSI.
  - d) Design and construction shall conform to the "Specifications for the Design, Fabrication and Erection of Structural Steel for Buildings" of the AISC unless otherwise shown or specified.
  - e) Connections: Unless otherwise detailed shall be standard connections using 3/4" diameter fasteners made in accordance with the latest edition of Steel Construction Manual of the American Institute of Steel Construction.
    - Shop Connections: May be welded or high tensile bolted, ASTM A325.
    - Field Connections: Shall be bolted with high tensile bolts, ASTM A325.

- JOIST AND JOIST GIRDERS:
- a) The design, fabrication, and erection of joists and joist girders shall conform to the Standard Specifications of the Steel Joist Institute.
  - b) Joists to be designed for wind uplift in conformance with Factory Mutual requirements. Reference: FM Data Sheet 1-7.
  - c) Point loads shown joists and joist girders are in addition to dead and live loads. Joist manufacturer to size joists and joist girders to carry additional loads.
- | Wind Uplift   | Gross           | Net             |
|---------------|-----------------|-----------------|
| Roof Interior | 15 lbs./sq. ft. | 8 lbs./sq. ft.  |
| Roof Edges    | 30 lbs./sq. ft. | 23 lbs./sq. ft. |
| Roof Corners  | 45 lbs./sq. ft. | 38 lbs./sq. ft. |

- GENERAL:
- a) Welding of steel roof deck shall provide resistance to design wind loads through diaphragm action and shall conform with Steel Roof Deck Institute Design Manual including side lap fastening.
  - b) Steel frame shall be gaged until all bracing and roof deck is installed and all connections are completed including roof deck welding.
  - c) All welding shall conform to the Structural Welding Code of the American Welding Society, latest edition.
  - d) Check mechanical and electrical drawings for exact location and size openings for equipment.

## FOUNDATION PLAN

- 1) T.F. DENOTES TOP OF FOOTING ELEVATION
- 2) T.F. EXTERIOR FOOTINGS (-)-0' U.N.O.
- 3) T.F. INTERIOR FOOTINGS (-)-1'-0" U.N.O.
- 4) COORDINATE FOOTING STEPS W/ TILT-UP ELEVATIONS

DATE	DESCRIPTION
12/19/94	RELEASED FOR CONSTRUCTION
11/21/94	STRUCTURAL REVISIONS
11/15/94	STRUCTURAL REVISIONS
10/21/94	SHELL APPROVAL

**Gray**  
 2500 Financial Center  
 250 W. Main Street  
 Lexington, Kentucky  
 [502] 281-5000

**James N. Gray Construction Co.**  
 Design - Engineering - Construction

101 Pin Oak Lane  
 P.O. Box 277  
 Glasgow, Kentucky  
 [502] 651-8891

**CONSTRUCTION DOCUMENT**

**MARTIN MILLS, INC.**  
**VIDALIA APPAREL DIVISION**  
**VIDALIA, LOUISIANA**

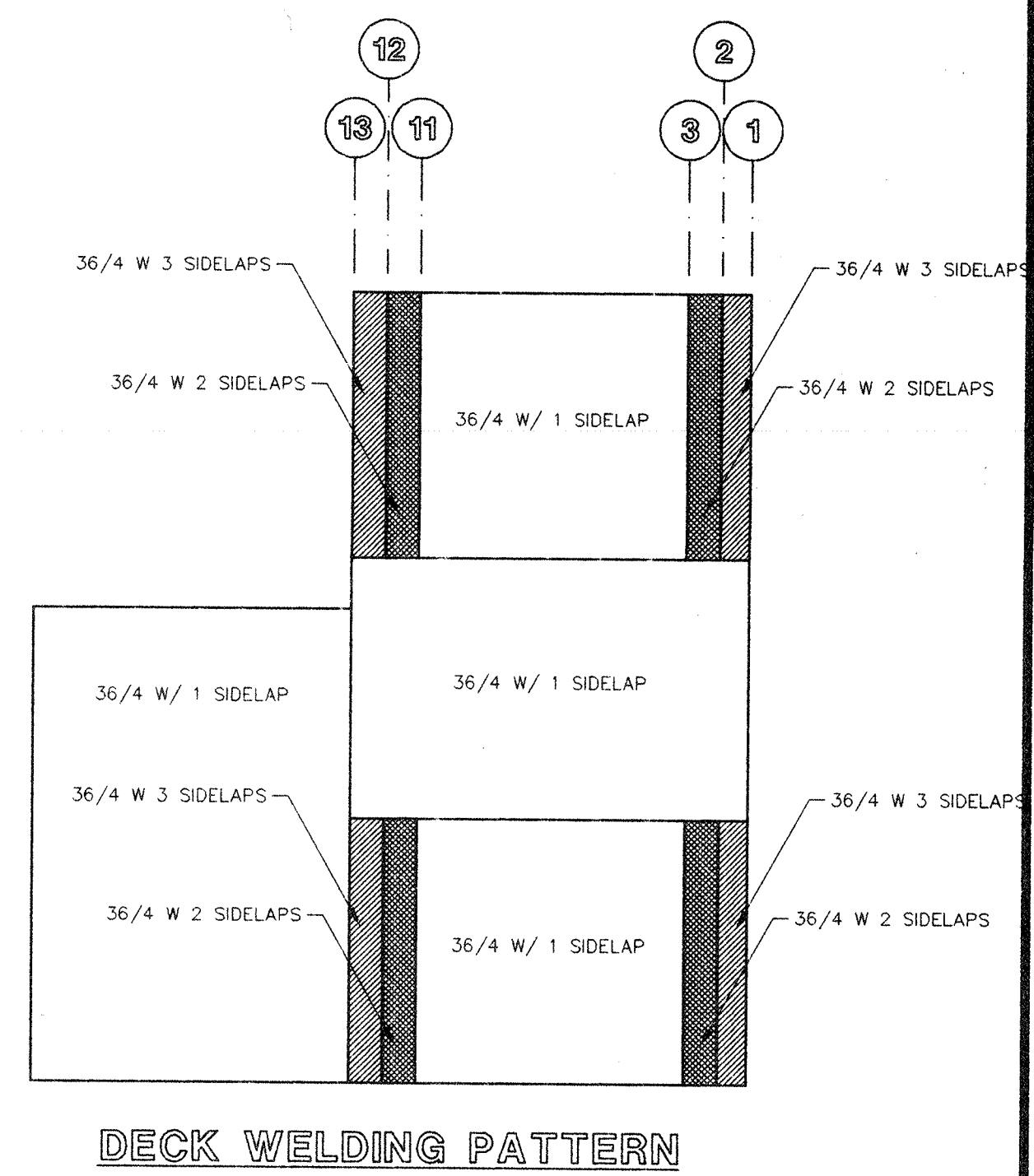
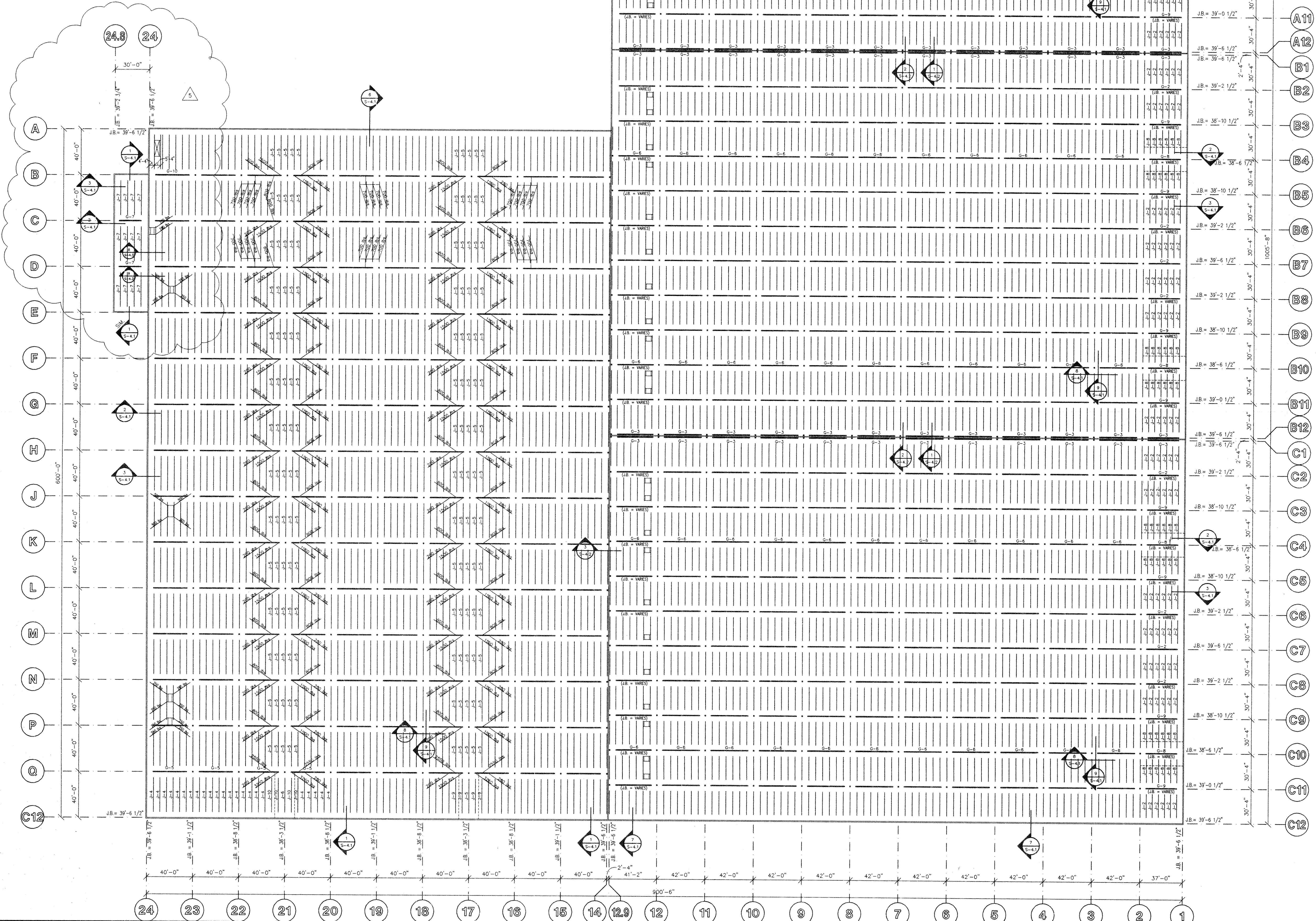
**FOUNDATION PLAN**

### JOIST SCHEDULE

MARK	DESCRIPTION	DISTANCE	P(1)	REMARKS
J-1	20K (192/120)	-	-	TYP. COLUMN LINES 1 TO 13 A1 TO C12 U.N.G.
J-2	20K (169/106)	-	-	-
J-3	22K (180/91)	-	-	TYP. COLUMN LINES 14 TO 24 A TO C12 U.N.G.
J-4	22K (180/91)	20'-0"	900 lbs.	-
J-5	22K (192/137)	-	-	-
J-6	22K (192/137)	20'-0"	900 lbs.	-
J-7	22K (169/96)	-	-	-
J-8	20K (169/106)	10'-6"	1000 lbs.	-
J-9	22K (279/224)	-	-	-
J-10	22K (279/224)	20'-0"	900 lbs.	-

### JOIST GIRDER SCHEDULE

MARK	DESCRIPTION	REMARKS
G-1	40G 7N 4.7k	TYP. COLUMN LINES 1 TO 13 A1 TO C12 U.N.G.
G-2	40G 7N 4.2k	-
G-3	40G 7N 2.4k	-
G-4	40G 7N 5.9k	TYP. COLUMN LINES 14 TO 24 A TO C12 U.N.G.
G-5	40G 7N 6.4k	-
G-6	40G 7N 5.6k	-
G-7	40G 5N 6.24k	-
G-8	40G 7N 5.5k	-
G-9	40G 7N 4.9k	-
G-10	-	SEE DETAIL ON S-2-3



### ROOF FRAMING PLAN

1) J.B. DENOTES JOIST BEARING ELEVATION  
 2) FURNISH 2 1/2" DEEP JOIST SEATS  
 3) FURNISH 1 1/2" MC22 STEEL ROOF DECK, PAINTED

Wind Uplift	Roofs	Net
Roof Interior	15 lbs./sq. ft.	8 lbs./sq. ft.
Roof Edges	30 lbs./sq. ft.	23 lbs./sq. ft.
Roof Corners	45 lbs./sq. ft.	38 lbs./sq. ft.

o = 16'-0"

DATE	DESCRIPTION
12/19/94	RELEASED FOR CONSTRUCTION
11/14/94	STRUCTURAL REVISIONS
10/21/94	SHELL APPROVAL

**Gray**  
 2500 Financial Center  
 250 W. Main Street  
 Glasgow, Kentucky  
 [502] 651-8891  
 [606] 281-5000

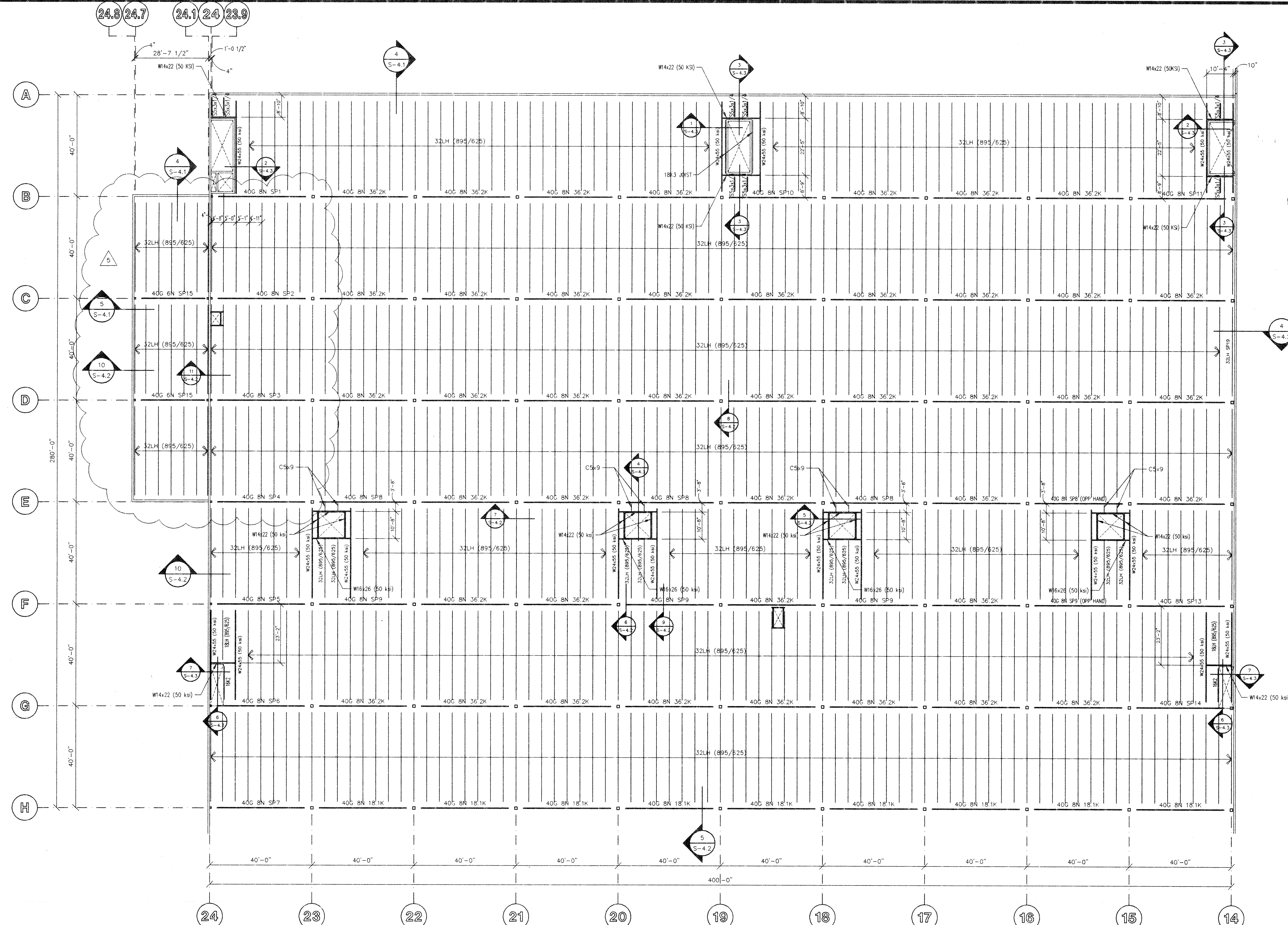
**James N. Gray Construction Co.**  
 Design - Engineering - Construction

**CONSTRUCTION DOCUMENT**

**MARTIN MILLS, INC.**  
**VIDALIA APPAREL DIVISION**  
**VIDALIA, LOUISIANA**

**ROOF FRAMING PLAN**

PLOT DATE 12/22/1994 - FK



### MEZZANINE FRAMING PLAN

- 1) TOP SLAB = 14'-12" x 40'-0"
- 2) 4" CONC. SLAB W/ 1.5% VL 22 GAGE COMPOSITE DECK ATTACH DECK TO SUPPORTS W/ HLT ENP2 ANCHORS ON A 36/4 PATTERN W/ NO SIDELAP FASTENERS
- 3) DESIGN LIVE LOAD 125 LB./SQ. FT.

### COLUMN SCHEDULE

TYPE	C1	C2	C3	C3A	C3B	C4	C5	C6	C7	C8	C9	C10	C11
ROOF	B-14 D-14 C-14 T-14 H-14 4	B-15 TO H-15 B-16 TO H-16 B-17 TO H-17 B-18 TO H-18 B-19 TO H-19 B-20 TO H-20 B-21 TO H-21 B-22 TO H-22 B-23 TO H-23	B-17 TO H-17 B-21 TO H-21	C-23.9 D-23.9 E-23.9 F-23.9 G-23.9 H-23.9 A-24.1 B-24.1 C-24.7 D-24.7	B-23.9	J-15 TO Q-15 J-16 TO Q-16 J-17 TO Q-17 J-18 TO Q-18 J-19 TO Q-19 J-20 TO Q-20 J-21 TO Q-21	J-17 TO Q-17 J-21 TO Q-21	J-14 TO Q-14	A2-2 TO I2 A3-2 TO I2 A4-2 TO I2 A5-2 TO I2 A6-2 TO I2 A7-2 TO I2 A8-2 TO I2 A9-2 TO I2 A10-2 TO I2 A11-2 TO I2 A12-2 TO I2 A13-2 TO I2 A14-2 TO I2 A15-2 TO I2 A16-2 TO I2 A17-2 TO I2 A18-2 TO I2 A19-2 TO I2 A20-2 TO I2 A21-2 TO I2 A22-2 TO I2 A23-2 TO I2 A24-2 TO I2 A25-2 TO I2 A26-2 TO I2 A27-2 TO I2 A28-2 TO I2 A29-2 TO I2 A30-2 TO I2 A31-2 TO I2 A32-2 TO I2 A33-2 TO I2 A34-2 TO I2 A35-2 TO I2 A36-2 TO I2 A37-2 TO I2 A38-2 TO I2 A39-2 TO I2 A40-2 TO I2 A41-2 TO I2 A42-2 TO I2 A43-2 TO I2 A44-2 TO I2 A45-2 TO I2 A46-2 TO I2 A47-2 TO I2 A48-2 TO I2 A49-2 TO I2 A50-2 TO I2 A51-2 TO I2 A52-2 TO I2 A53-2 TO I2 A54-2 TO I2 A55-2 TO I2 A56-2 TO I2 A57-2 TO I2 A58-2 TO I2 A59-2 TO I2 A60-2 TO I2 A61-2 TO I2 A62-2 TO I2 A63-2 TO I2 A64-2 TO I2 A65-2 TO I2 A66-2 TO I2 A67-2 TO I2 A68-2 TO I2 A69-2 TO I2 A70-2 TO I2 A71-2 TO I2 A72-2 TO I2 A73-2 TO I2 A74-2 TO I2 A75-2 TO I2 A76-2 TO I2 A77-2 TO I2 A78-2 TO I2 A79-2 TO I2 A80-2 TO I2 A81-2 TO I2 A82-2 TO I2 A83-2 TO I2 A84-2 TO I2 A85-2 TO I2 A86-2 TO I2 A87-2 TO I2 A88-2 TO I2 A89-2 TO I2 A90-2 TO I2 A91-2 TO I2 A92-2 TO I2 A93-2 TO I2 A94-2 TO I2 A95-2 TO I2 A96-2 TO I2 A97-2 TO I2 A98-2 TO I2 A99-2 TO I2 A100-2 TO I2	C2-2 TO I2 C3-2 TO I2 C4-2 TO I2 C5-2 TO I2 C6-2 TO I2 C7-2 TO I2 C8-2 TO I2 C9-2 TO I2 C10-2 TO I2 C11-2 TO I2	A4-2 TO I2 A10-2 TO I2 B4-2 TO I2 B10-2 TO I2 C4-2 TO I2 C10-2 TO I2	A12-2 TO I2 B1-2 TO I2 B12-2 TO I2 C1-2 TO I2	B4-13 B5-13 B6-13 B7-13 B8-13 B9-13 B10-13 B11-13 B12-13 B13-13 B14-13 B15-13 B16-13 B17-13 B18-13 B19-13 B20-13 B21-13 B22-13 B23-13 B24-13 B25-13 B26-13 B27-13 B28-13 B29-13 B30-13 B31-13 B32-13 B33-13 B34-13 B35-13 B36-13 B37-13 B38-13 B39-13 B40-13 B41-13 B42-13 B43-13 B44-13 B45-13 B46-13 B47-13 B48-13 B49-13 B50-13 B51-13 B52-13 B53-13 B54-13 B55-13 B56-13 B57-13 B58-13 B59-13 B60-13 B61-13 B62-13 B63-13 B64-13 B65-13 B66-13 B67-13 B68-13 B69-13 B70-13 B71-13 B72-13 B73-13 B74-13 B75-13 B76-13 B77-13 B78-13 B79-13 B80-13 B81-13 B82-13 B83-13 B84-13 B85-13 B86-13 B87-13 B88-13 B89-13 B90-13 B91-13 B92-13 B93-13 B94-13 B95-13 B96-13 B97-13 B98-13 B99-13 B100-13
MEZZANINE	IS12X12X3/8	IS12X12X3/8	IS12X12X3/8	IS7X7X1/2	IS7X7X1/2	IS8X8X1/4	IS8X8X1/4	IS8X8X1/4	IS8X8X1/4	IS8X8X1/4	IS8X8X1/4	IS8X8X1/4	IS8X8X1/4
FLOOR	0'-10"	10'	2'-4"	0'-10"	4'-10"	10'	2'-4"	2'-10"	10'	10'	2'-5"	2'-10"	2'-10"
BASE PLATE	18x16x1 1/4	18x18x1 1/8	18x18x1 1/8	14x11x1 3/8	14x11x1 3/8	14x14x3/4	14x14x3/4	14x14x3/4	14x14x3/4	14x14x3/4	14x14x3/4	14x14x3/4	14x14x3/4
ANCHOR BOLTS	4 - 3/4"	4 - 3/4"	4 - 3/4"	4 - 3/4"	4 - 3/4"	4 - 3/4"	4 - 3/4"	4 - 3/4"	4 - 3/4"	4 - 3/4"	4 - 3/4"	4 - 3/4"	4 - 3/4"

DATE	DESCRIPTION
12/19/94	RELEASED FOR CONSTRUCTION
11/21/94	STRUCTURAL REVISIONS
11/14/94	STRUCTURAL REVISIONS
10/21/94	SHELL APPROVAL

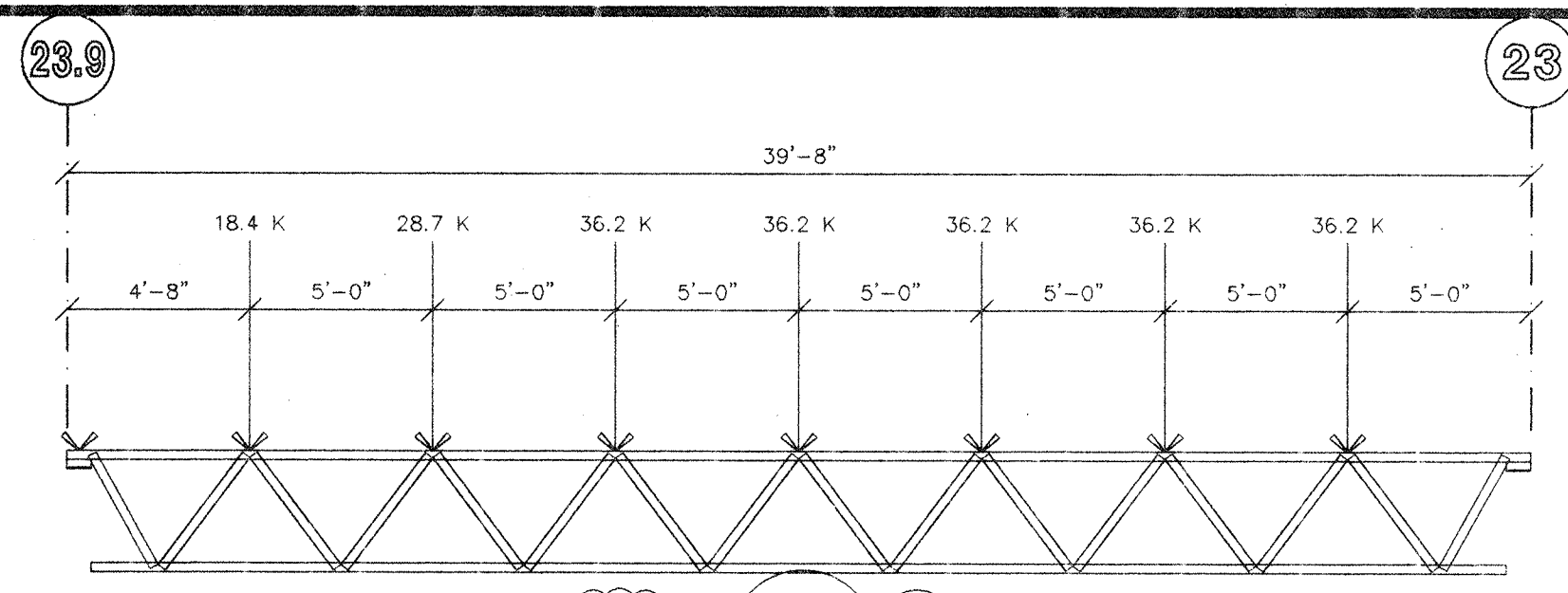
**Gray**  
 2500 Financial Center  
 P.O. Box 277  
 Glasgow, Kentucky  
 [502] 651-8891

**James N. Gray Construction Co.**  
 Design - Engineering - Construction

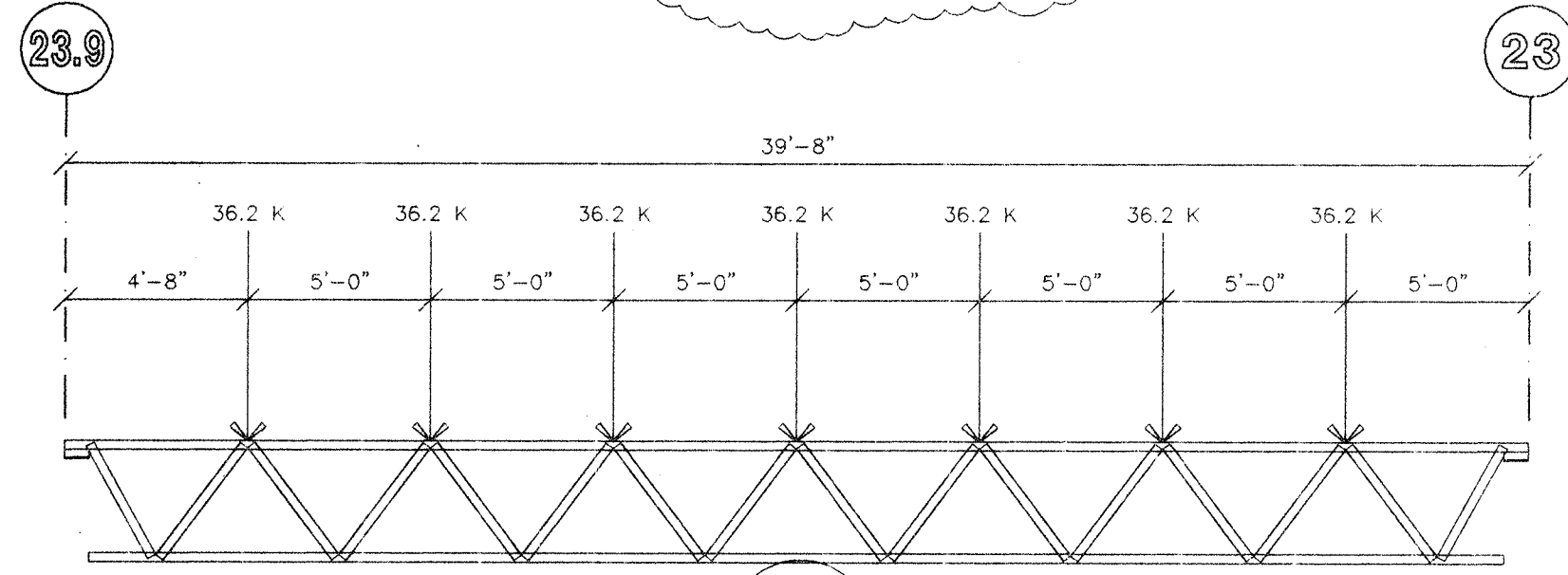
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**MARTIN MILLS, INC.**  
**VIDALIA APPAREL DIVISION**  
**VIDALIA, LOUISIANA**

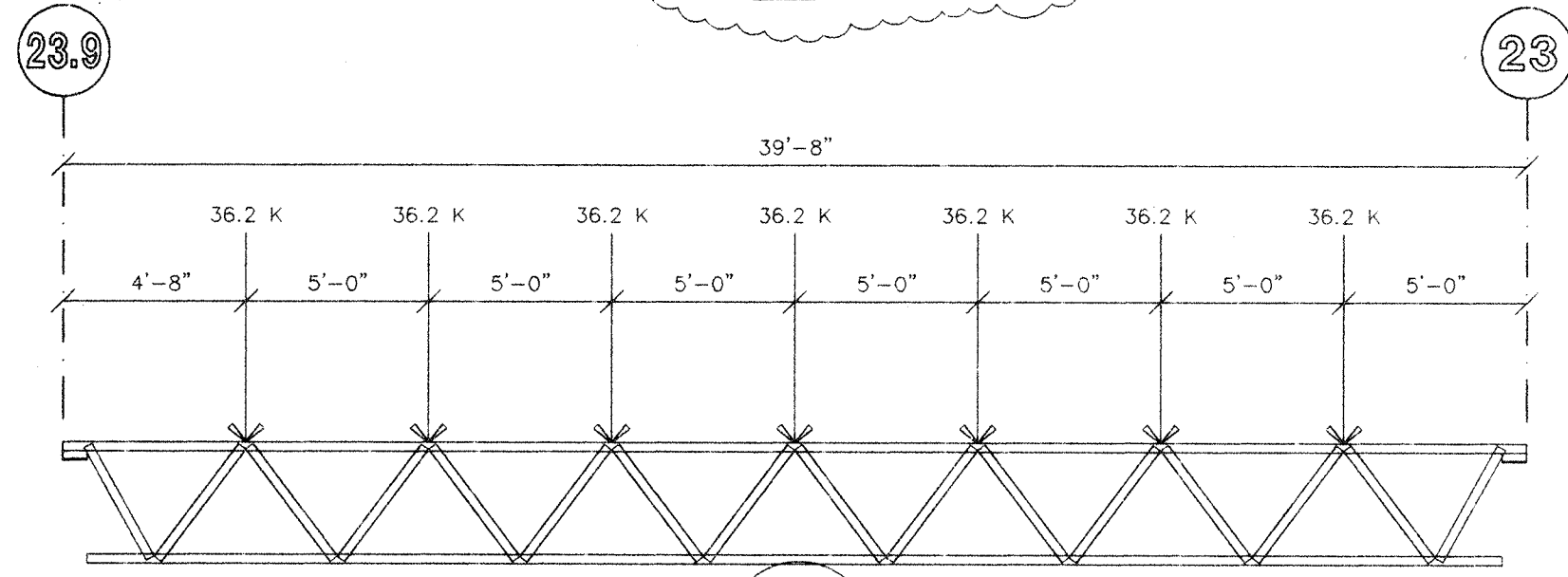
**MEZZANINE FRAMING PLAN**



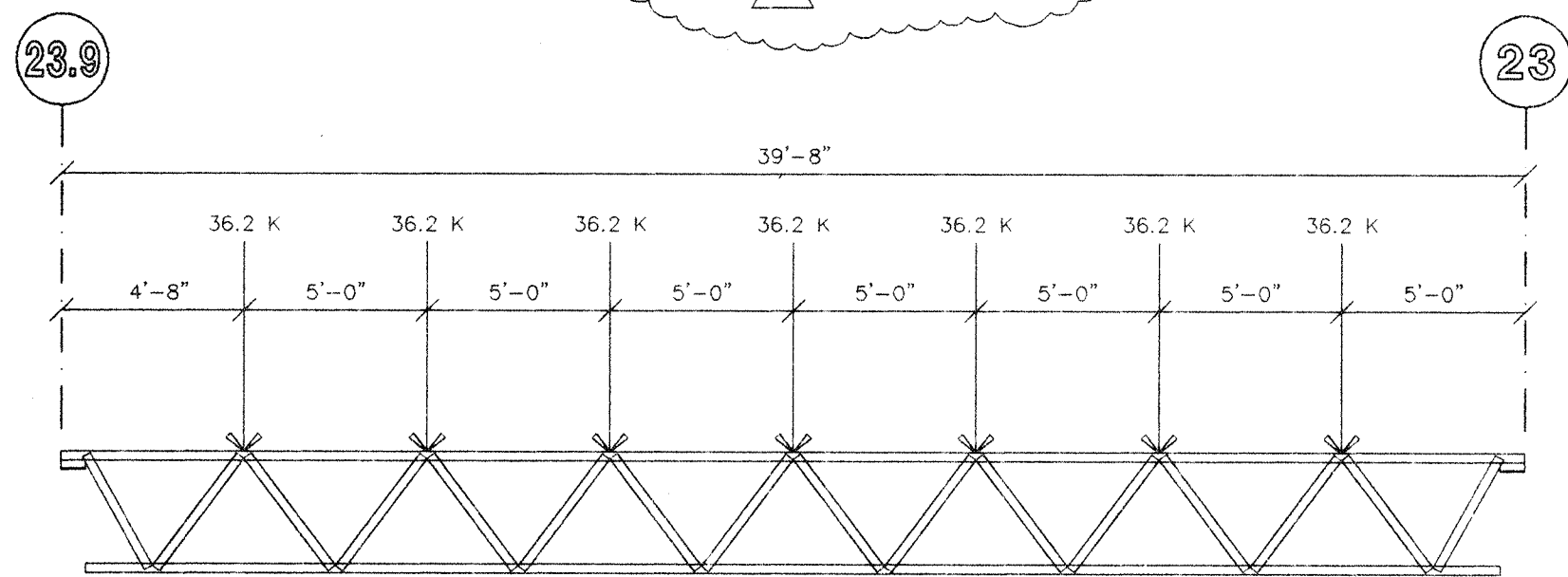
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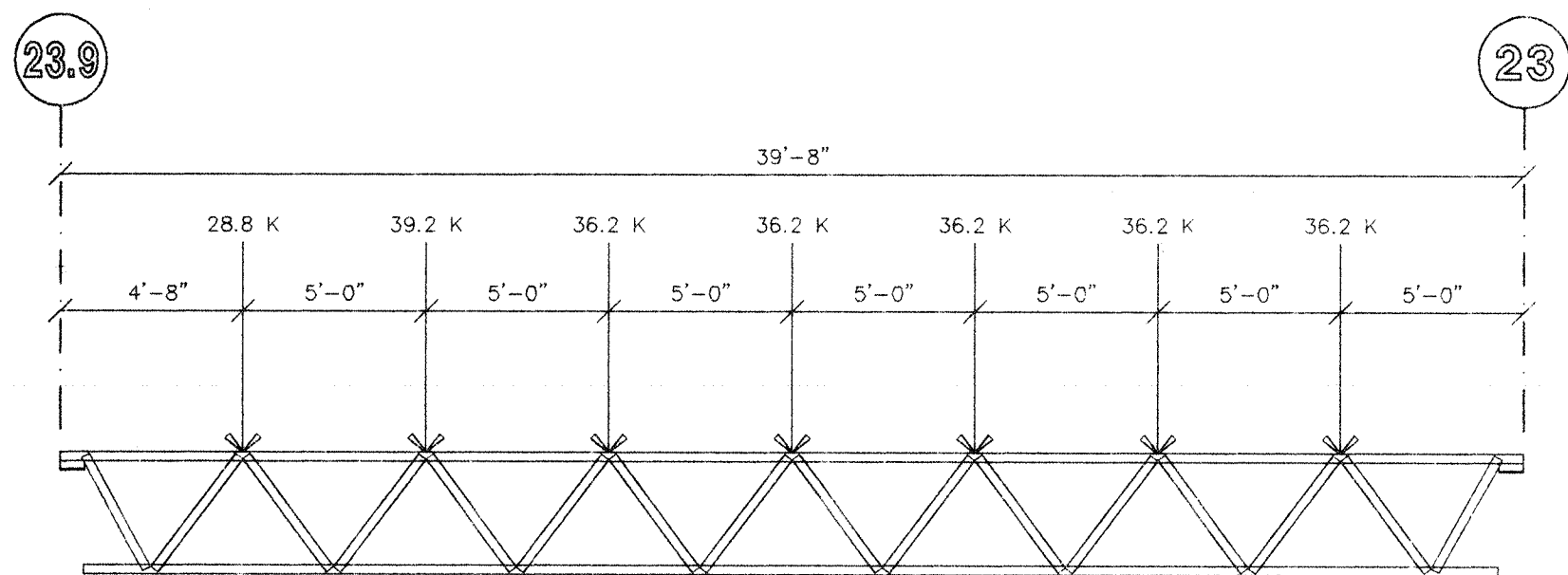
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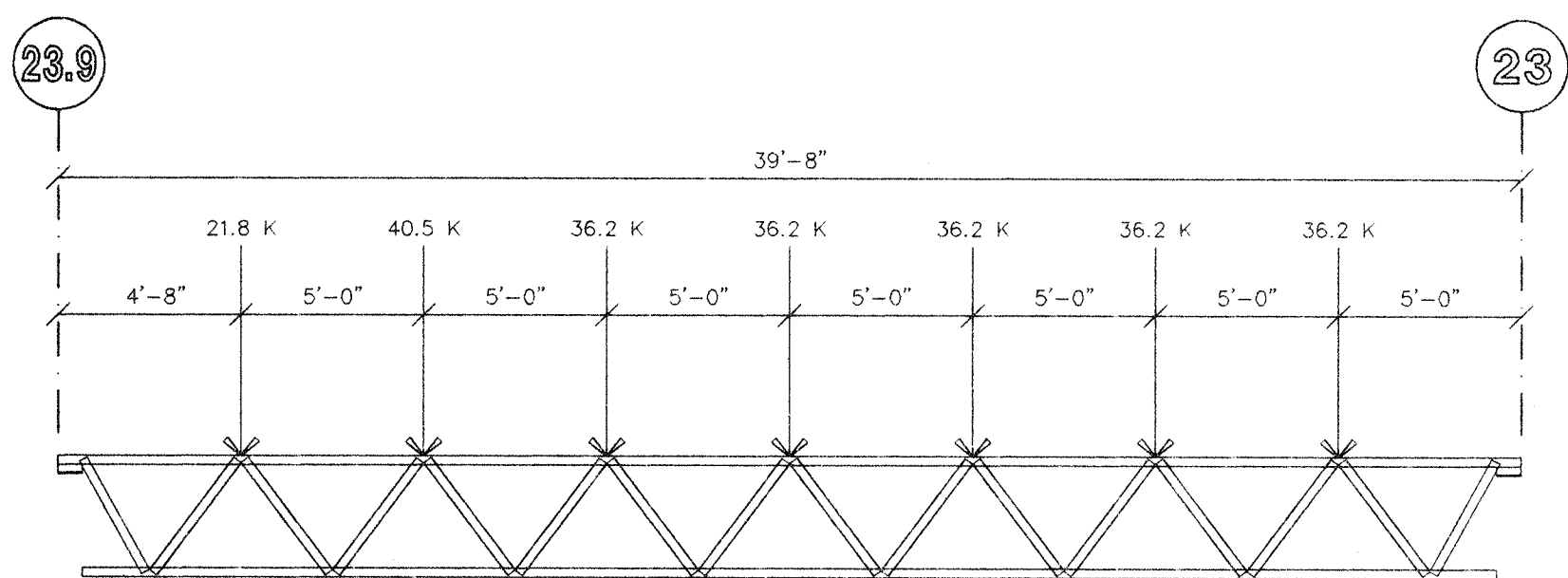
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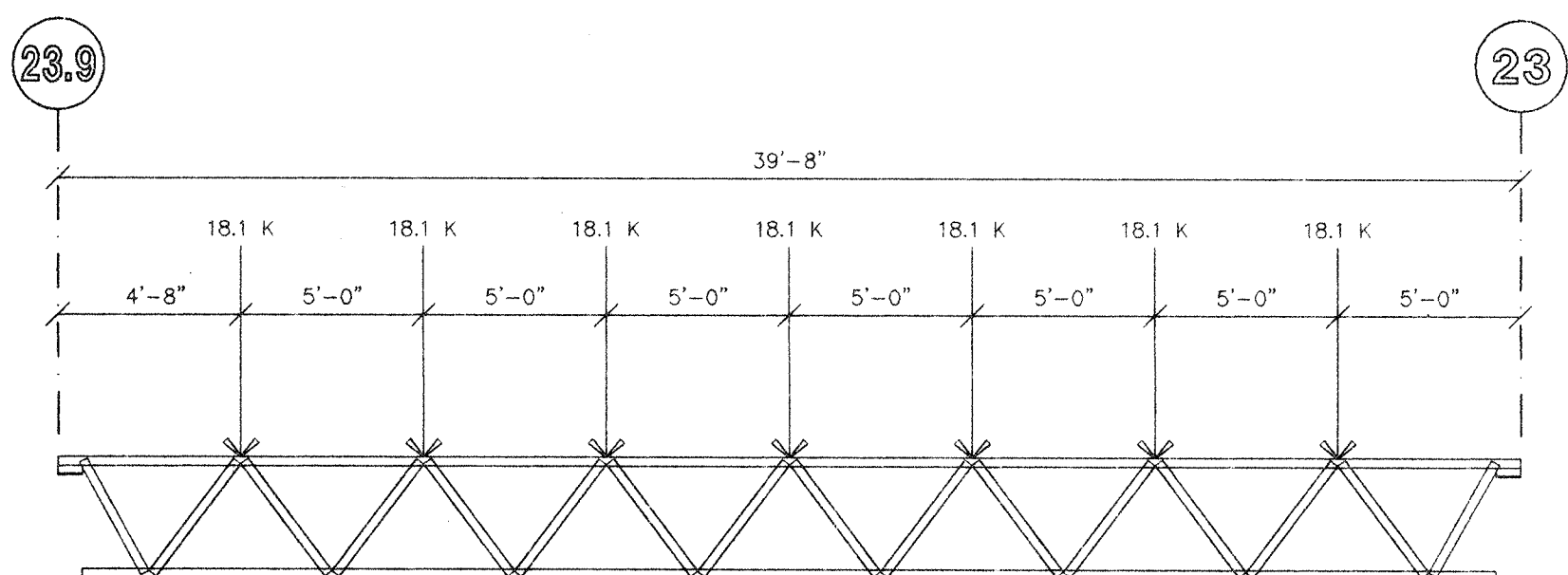
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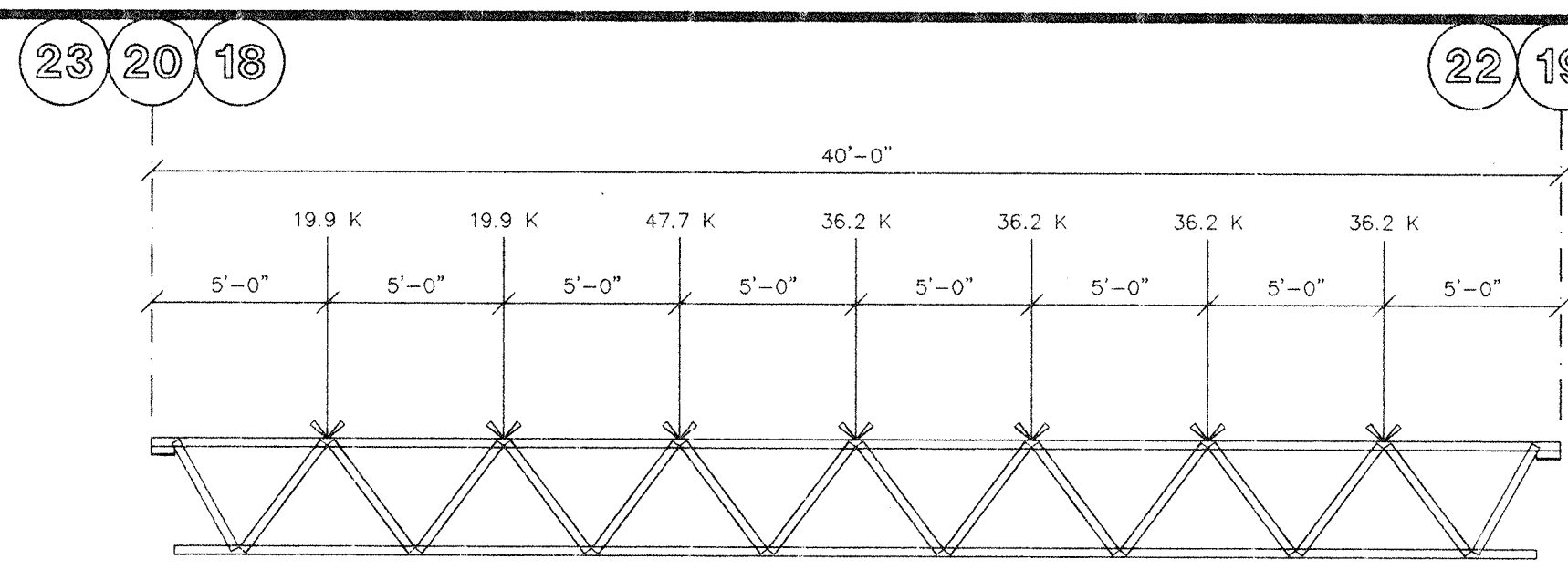
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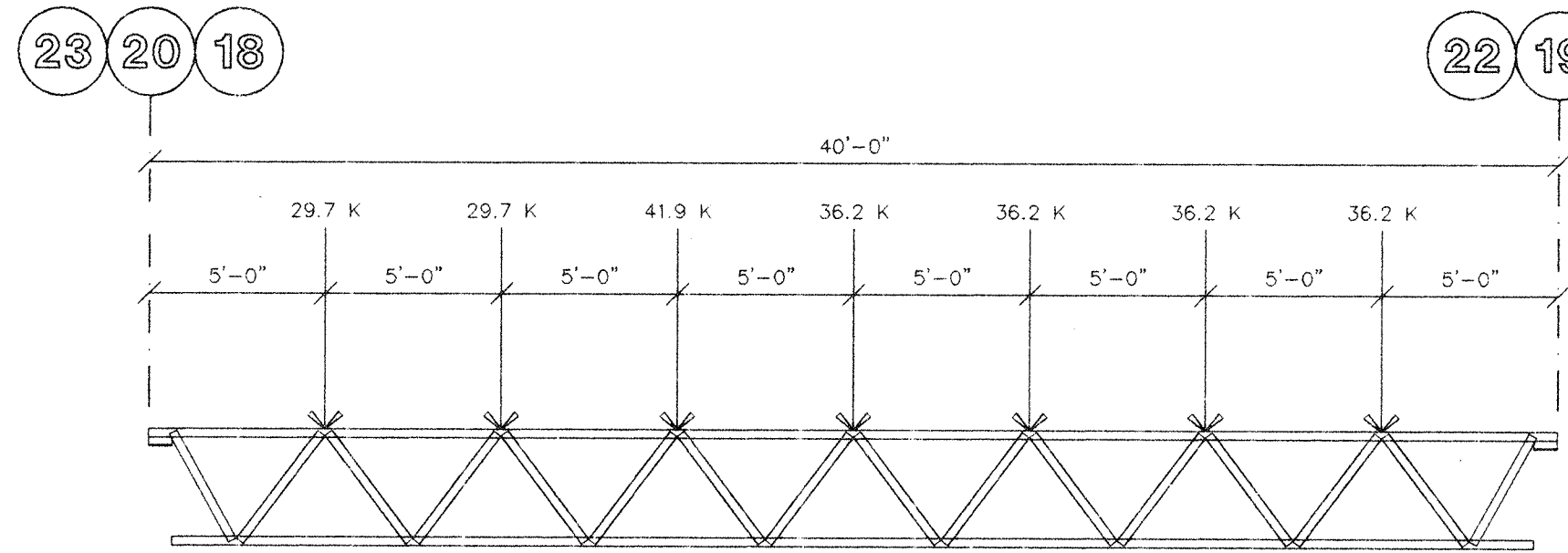
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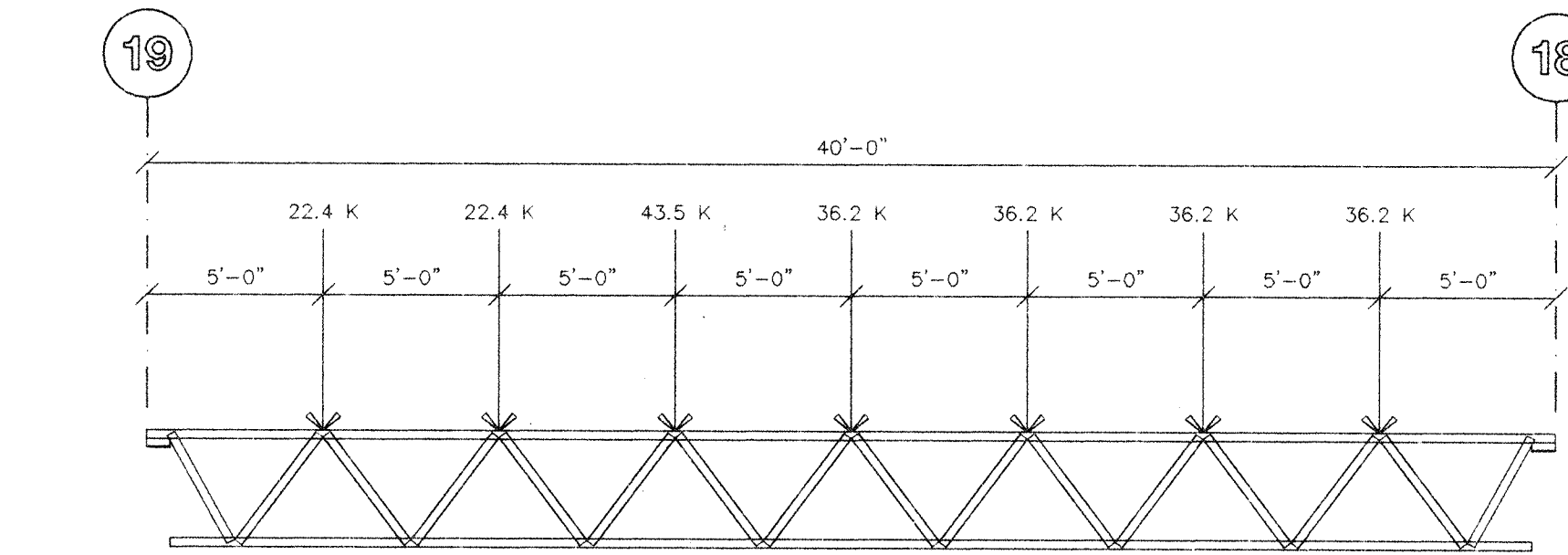
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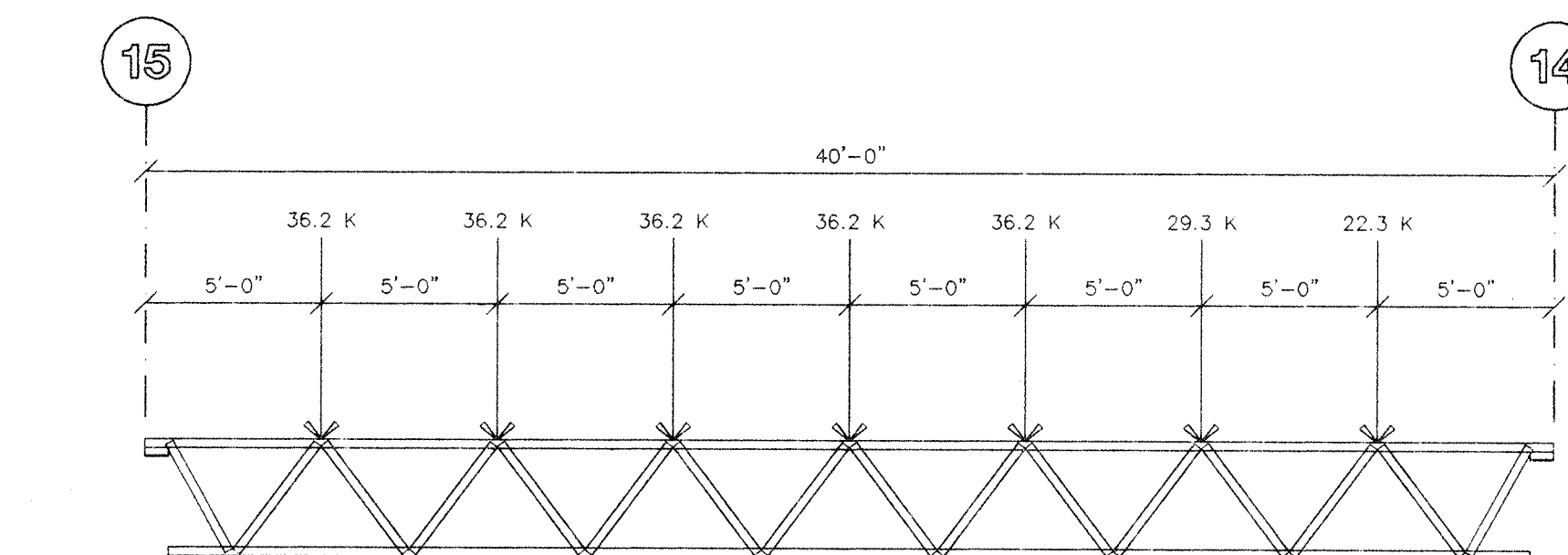
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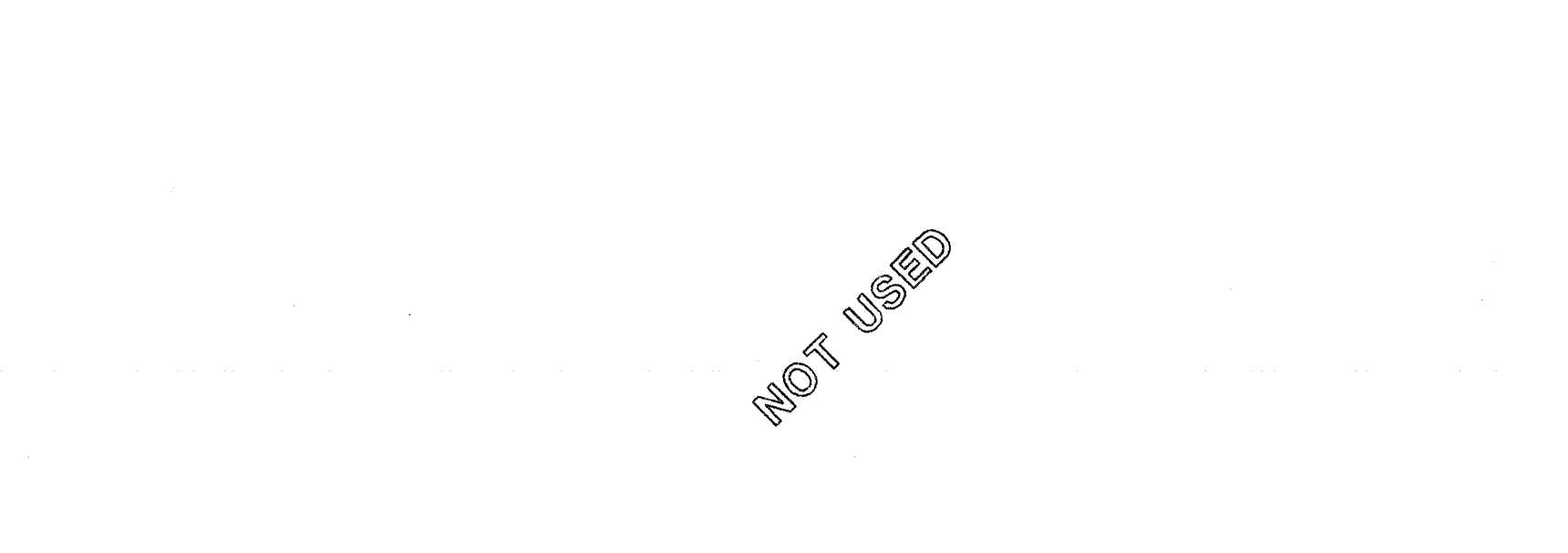
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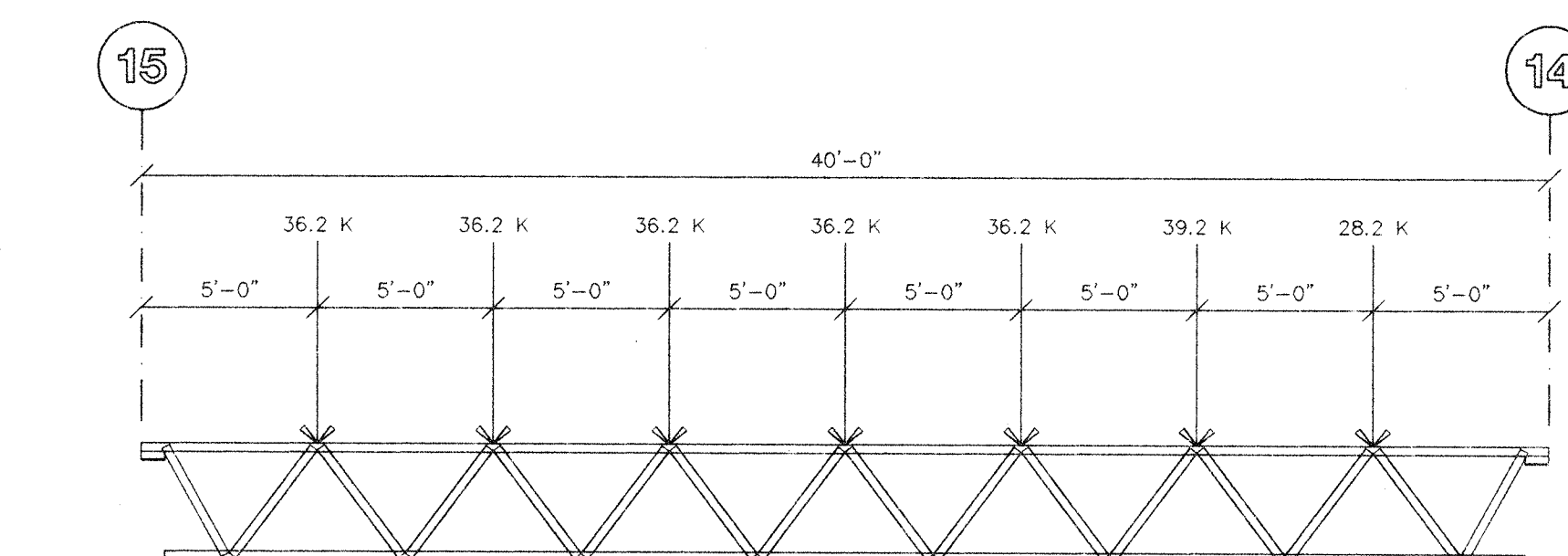
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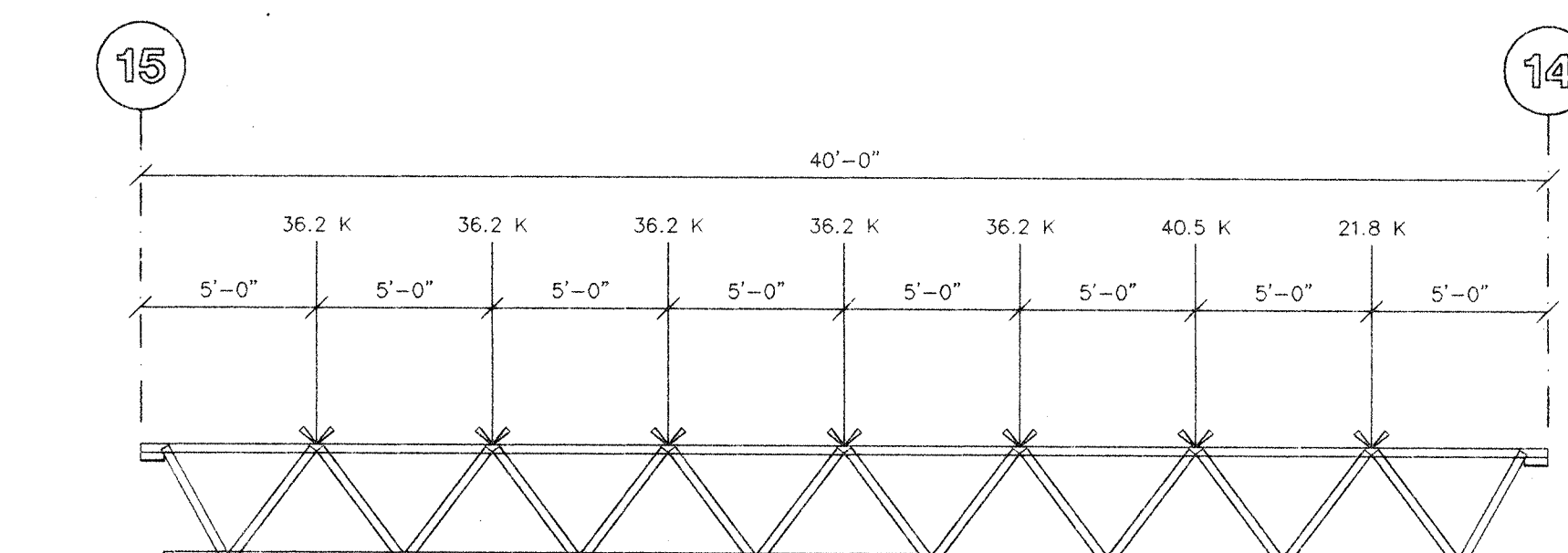
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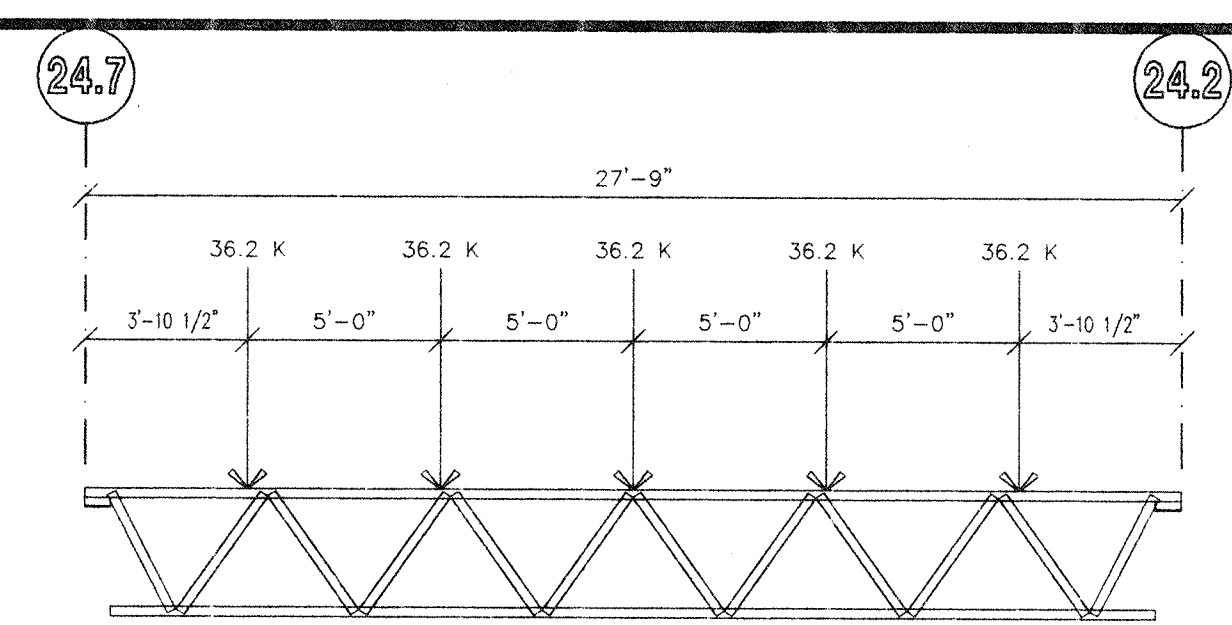
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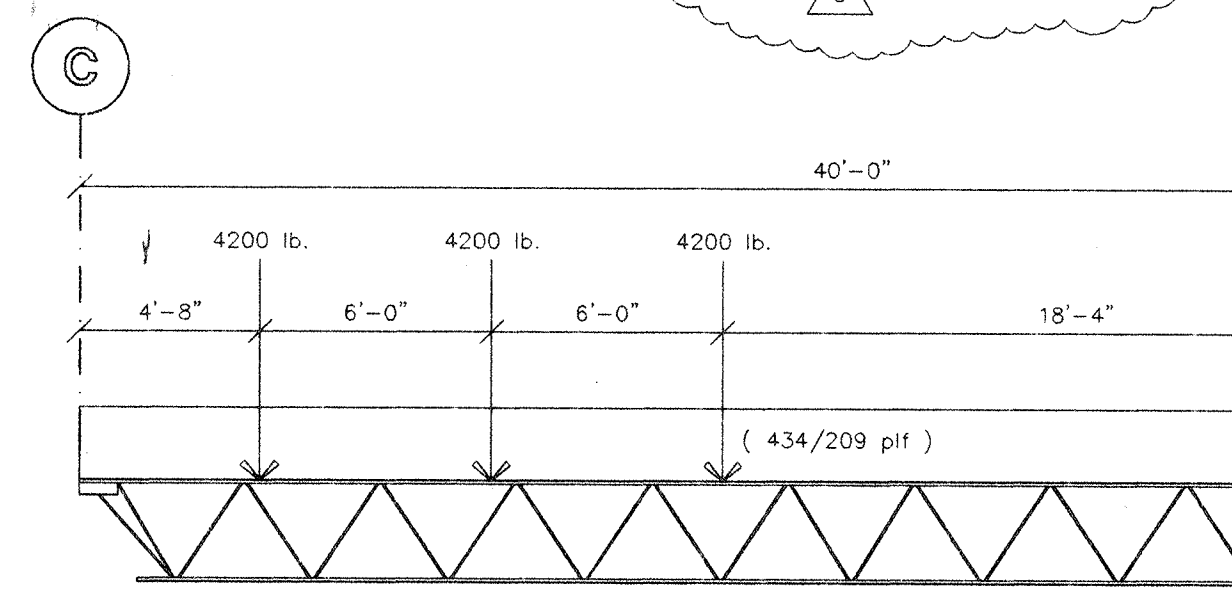
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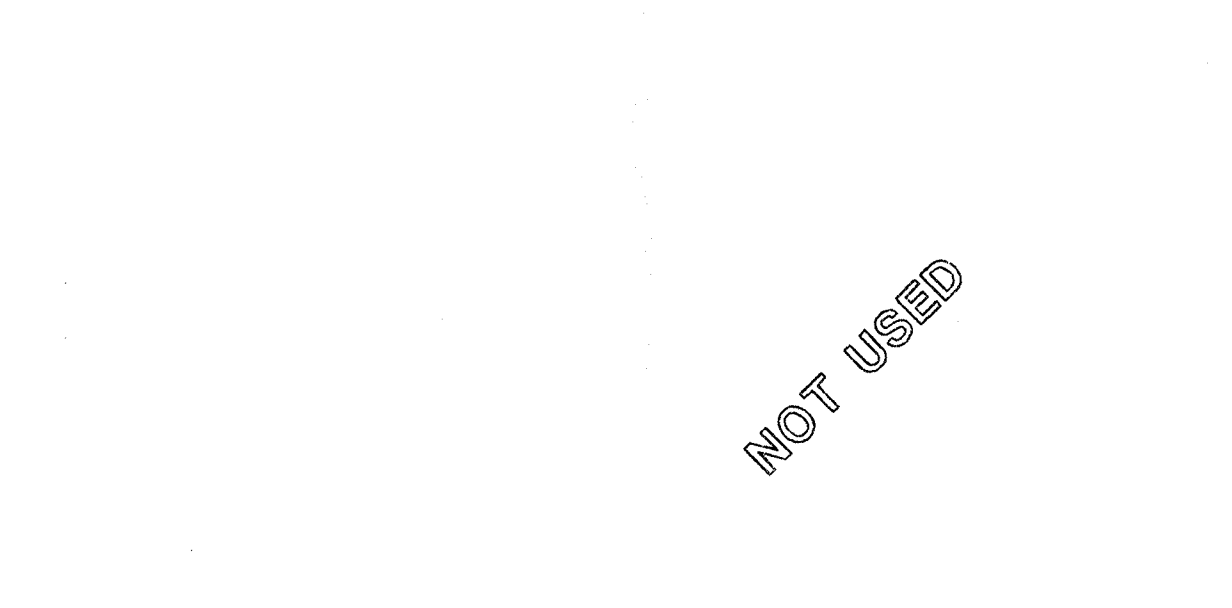
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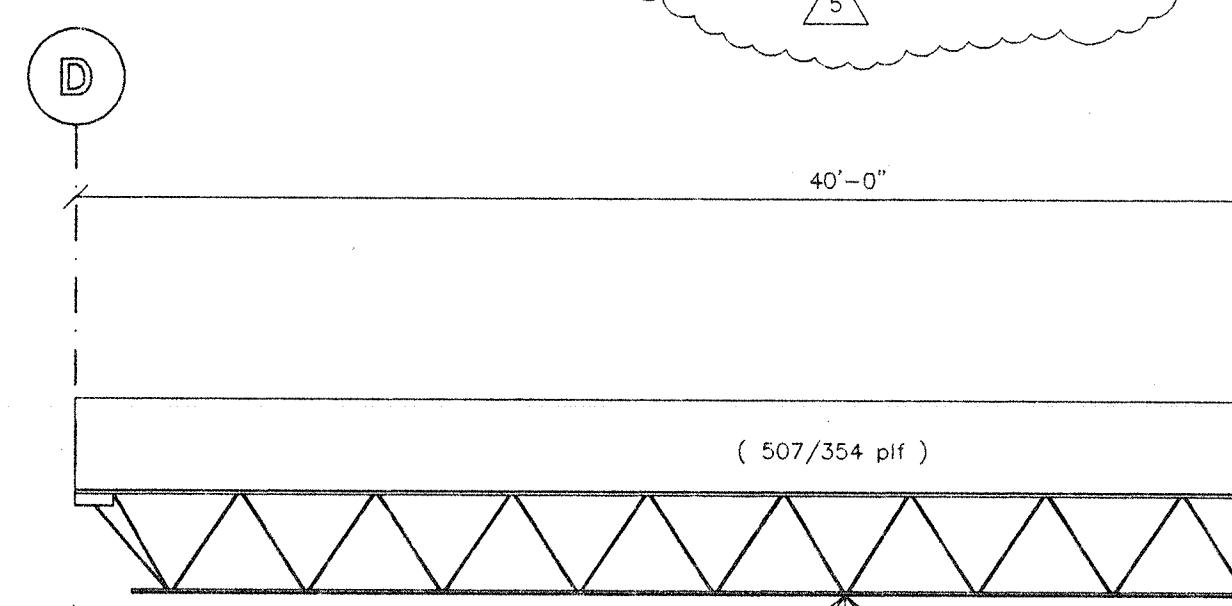
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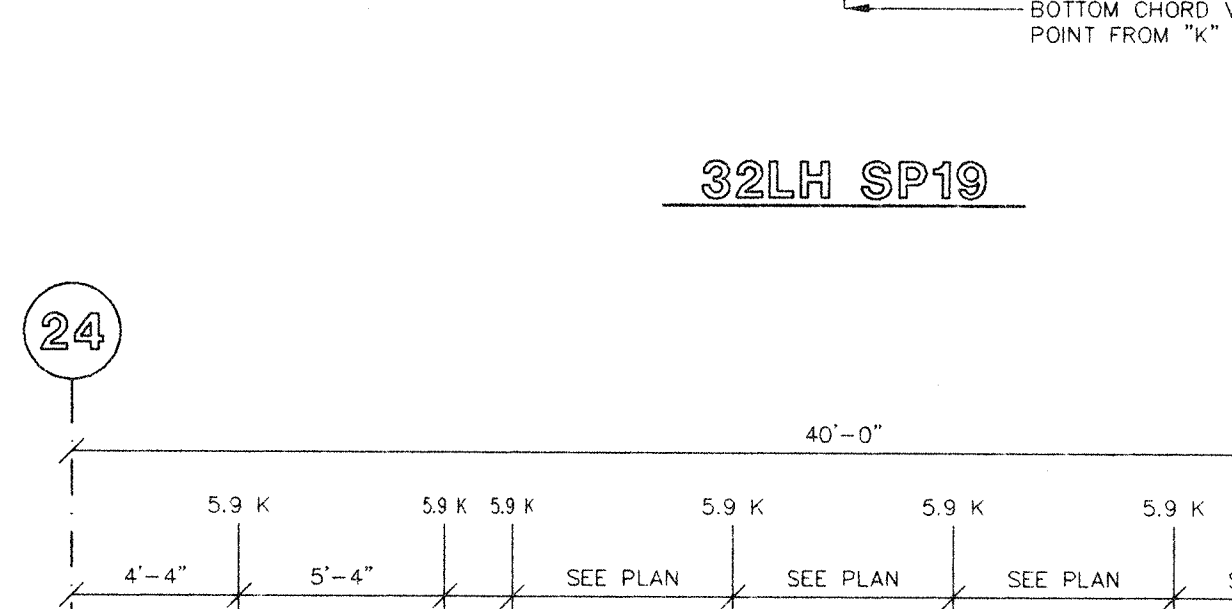
32LH SP17



32LH SP18



32LH SP19



G10

NOT USED

NOT USED

NOT USED

BOTTOM CHORD VERTICAL SUPPORT POINT FROM "K" BRACE BELOW

DATE	DESCRIPTION
12/19/94	RELEASED FOR CONSTRUCTION
11/21/94	STRUCTURAL REVISIONS
11/14/94	STRUCTURAL REVISIONS

**Gray**  
 2500 Financial Center  
 250 W. Main Street  
 Glasgow, Kentucky  
 [502] 651-8891 [606] 281-5000

**James N. Gray Construction Co.**  
 Design - Engineering - Construction

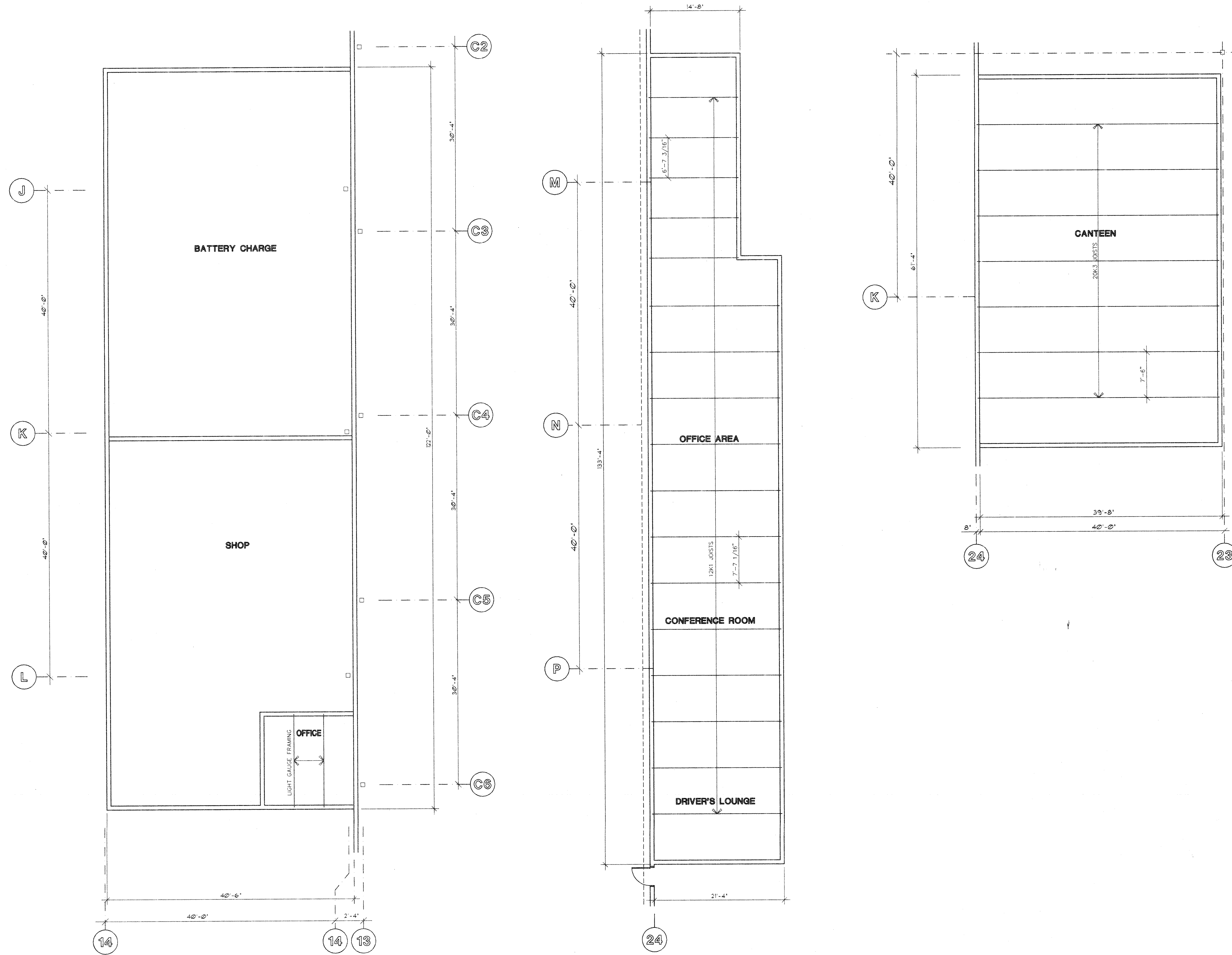
CONSTRUCTION DOCUMENT

MARTIN MILLS, INC.  
 VIDALIA APPAREL DIVISION  
 VIDALIA, LOUISIANA

MEZZANINE LOAD DIAGRAMS

JOB NUMBER	SHEET NUMBER
95067	G-23

PLOT DATE 12/22/1994 - PK



**DUST COVER FRAMING PLAN**

J.B. DENOTES JOIST BEARING ELEVATION  
 FURNISH 2 1/2" DEEP JOIST SEATS  
 FURNISH 1 1/2" MR22 STEEL ROOF DECK, PAINTED

DATE	DESCRIPTION
12/19/94	RELEASED FOR CONSTRUCTION
11/14/94	STRUCTURAL REVISIONS

**Gray**  
 101 Pin Oak Lane  
 P.O. Box 277  
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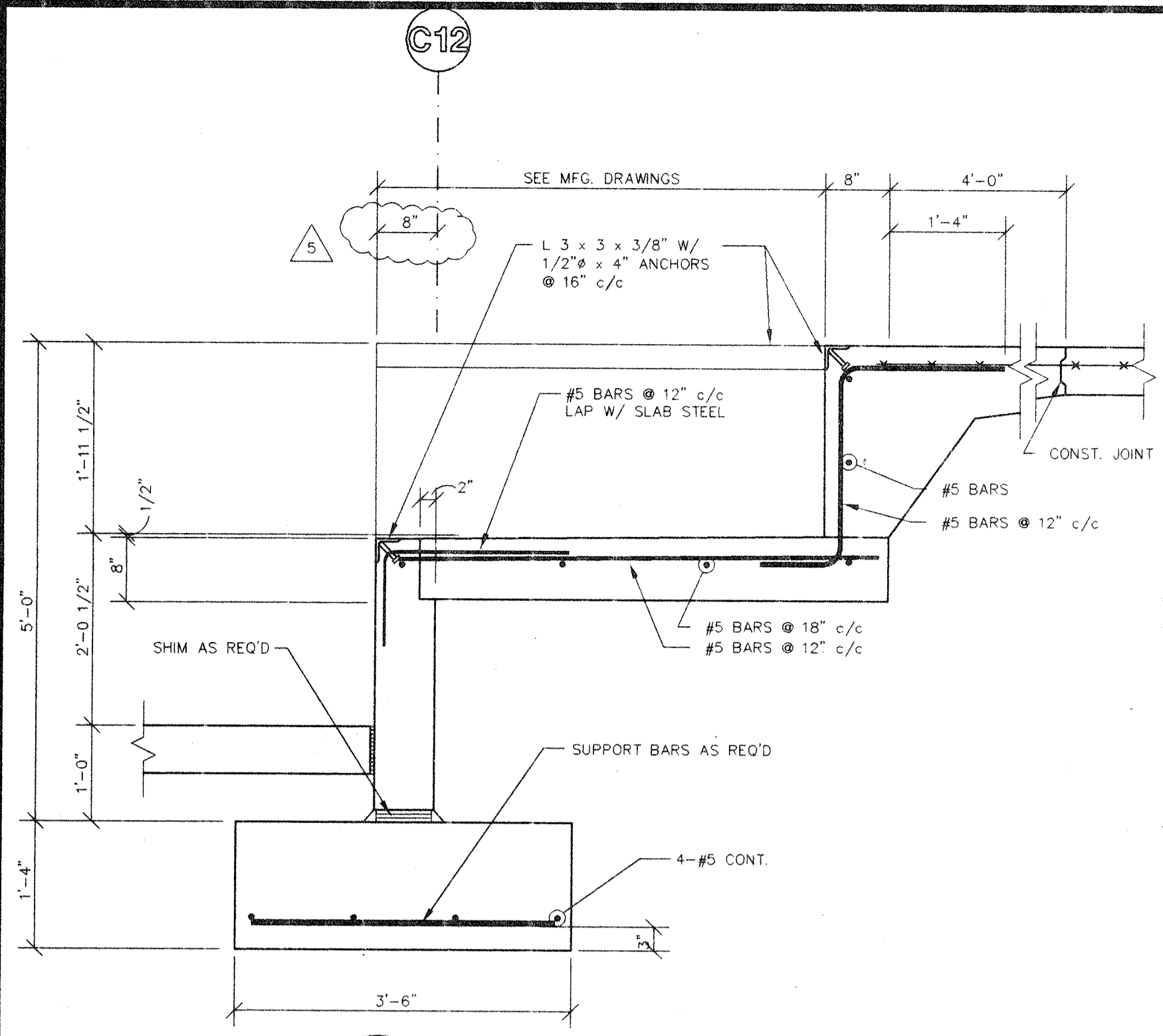
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**CONSTRUCTION DOCUMENT**

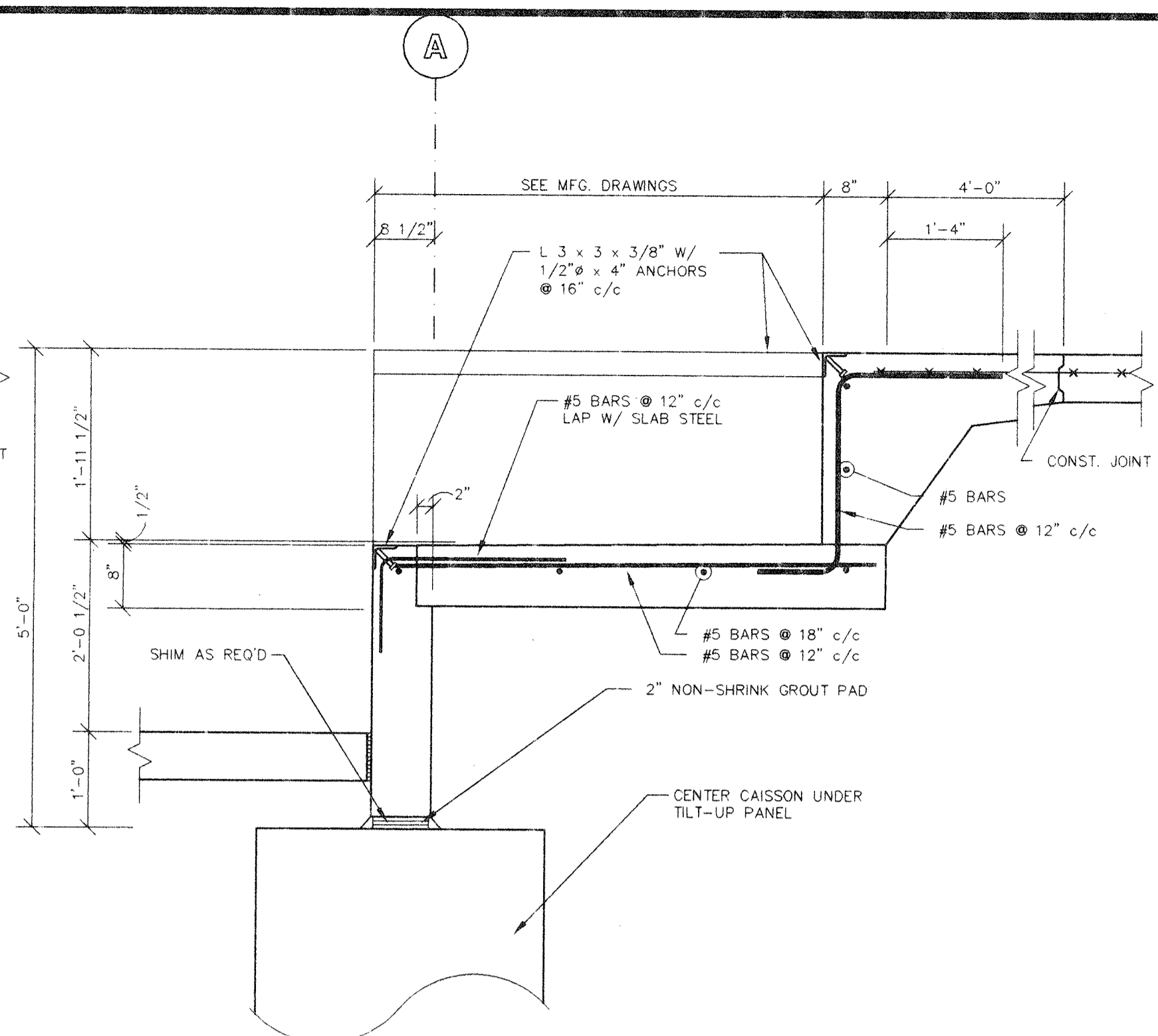
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**VIDALIA APPAREL DIVISION**  
**VIDALIA, LOUISIANA**

**DUST COVER FRAMING PLAN**

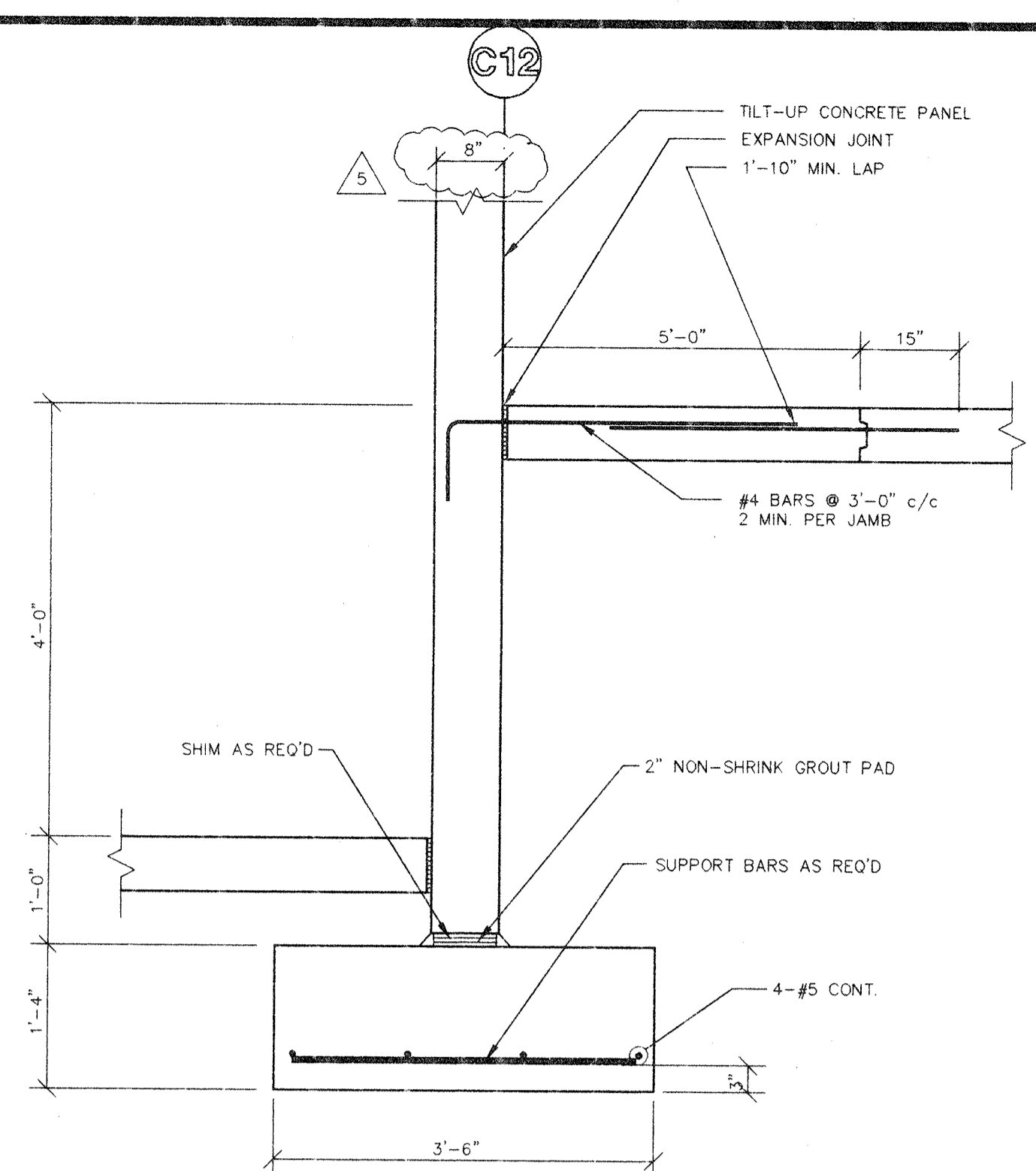
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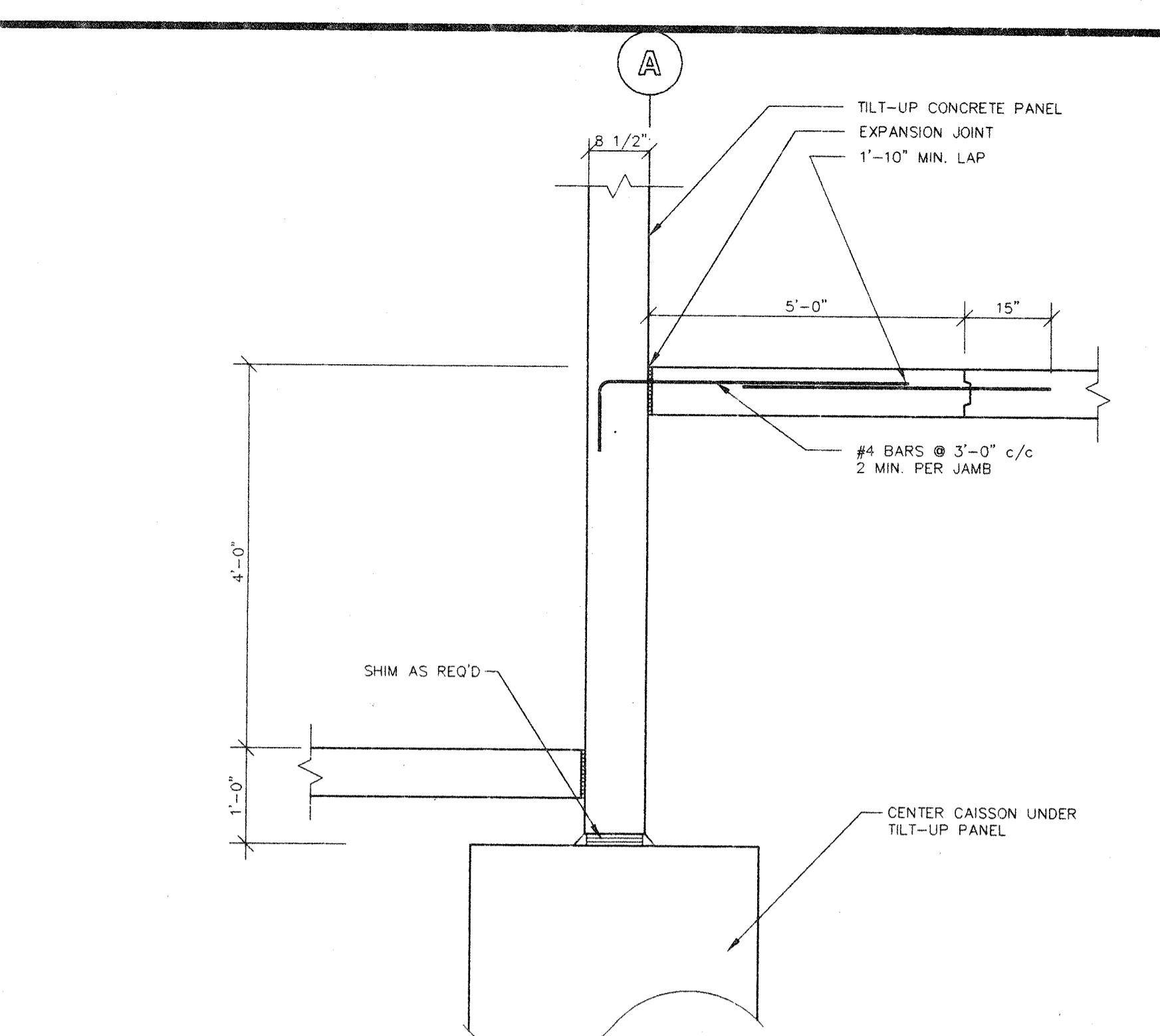
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S-3.1



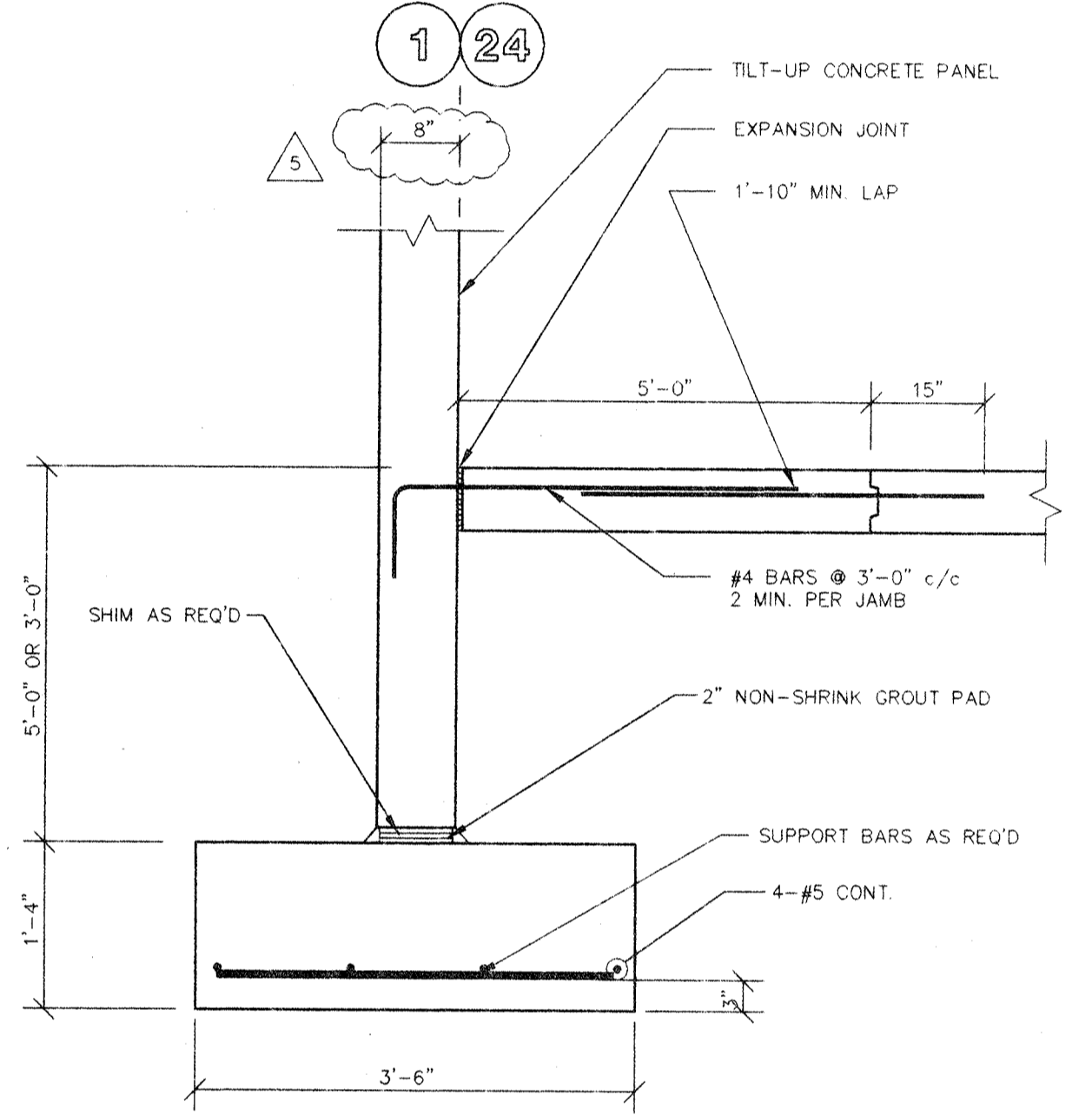
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S-3.1



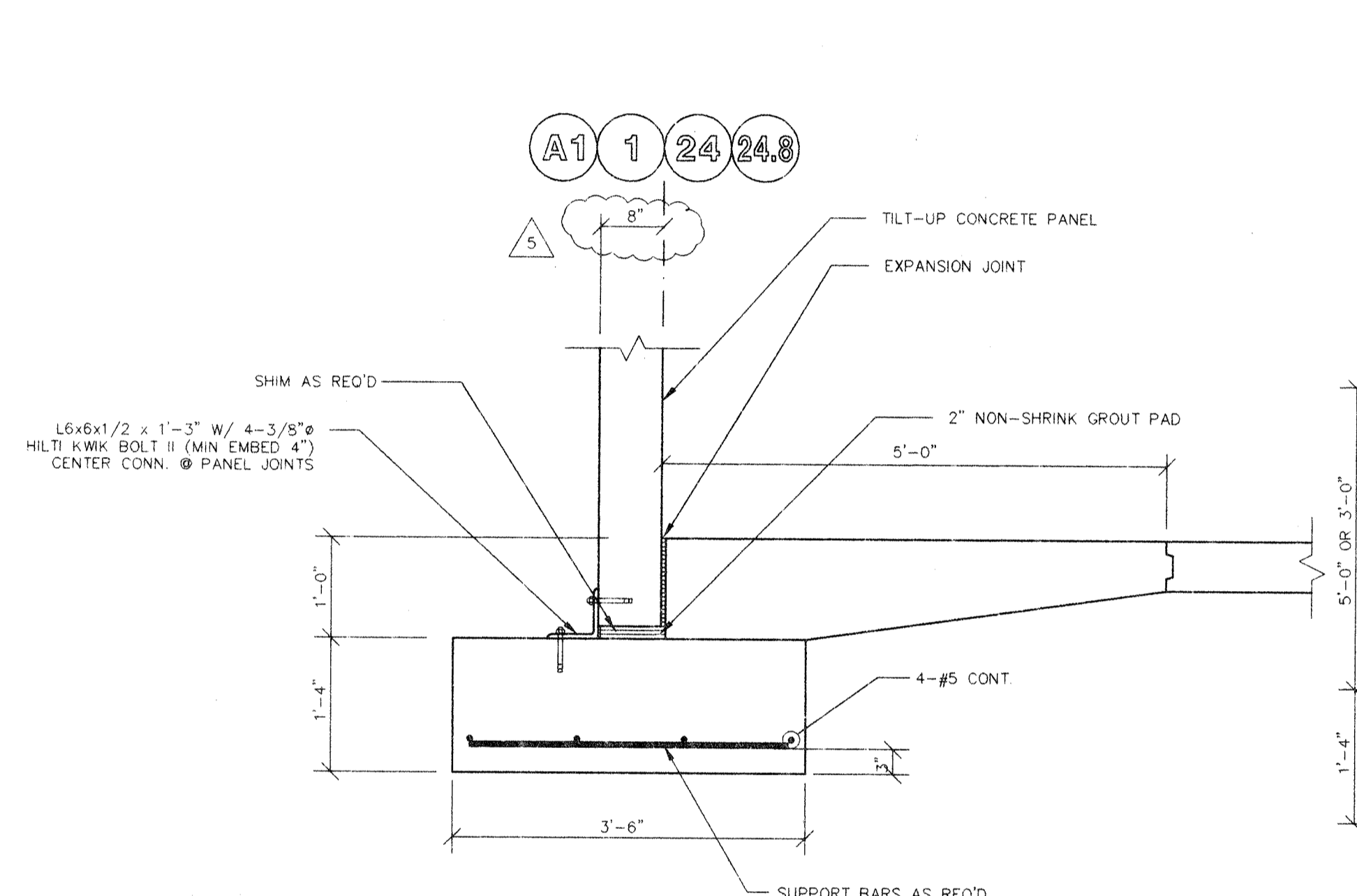
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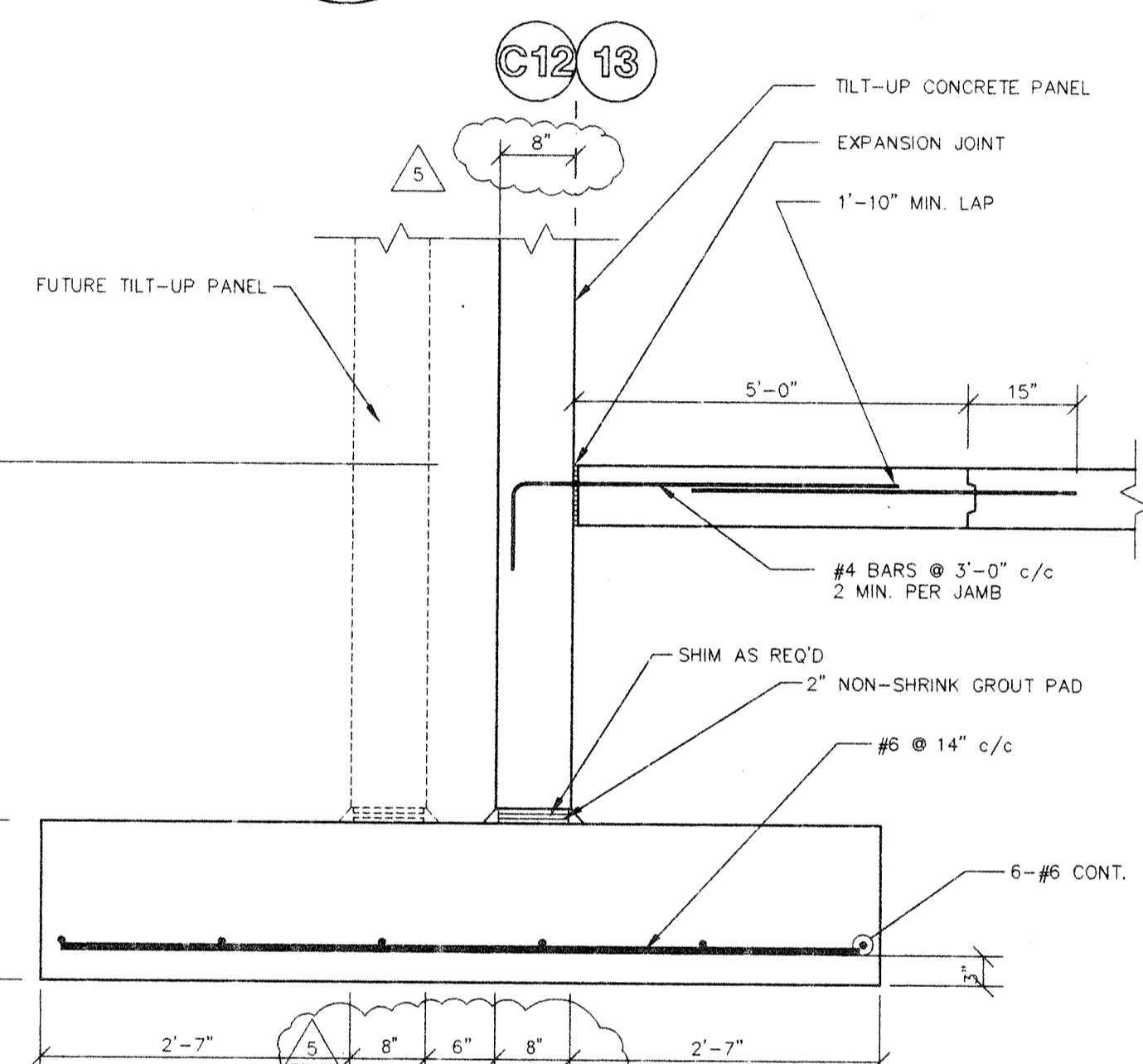
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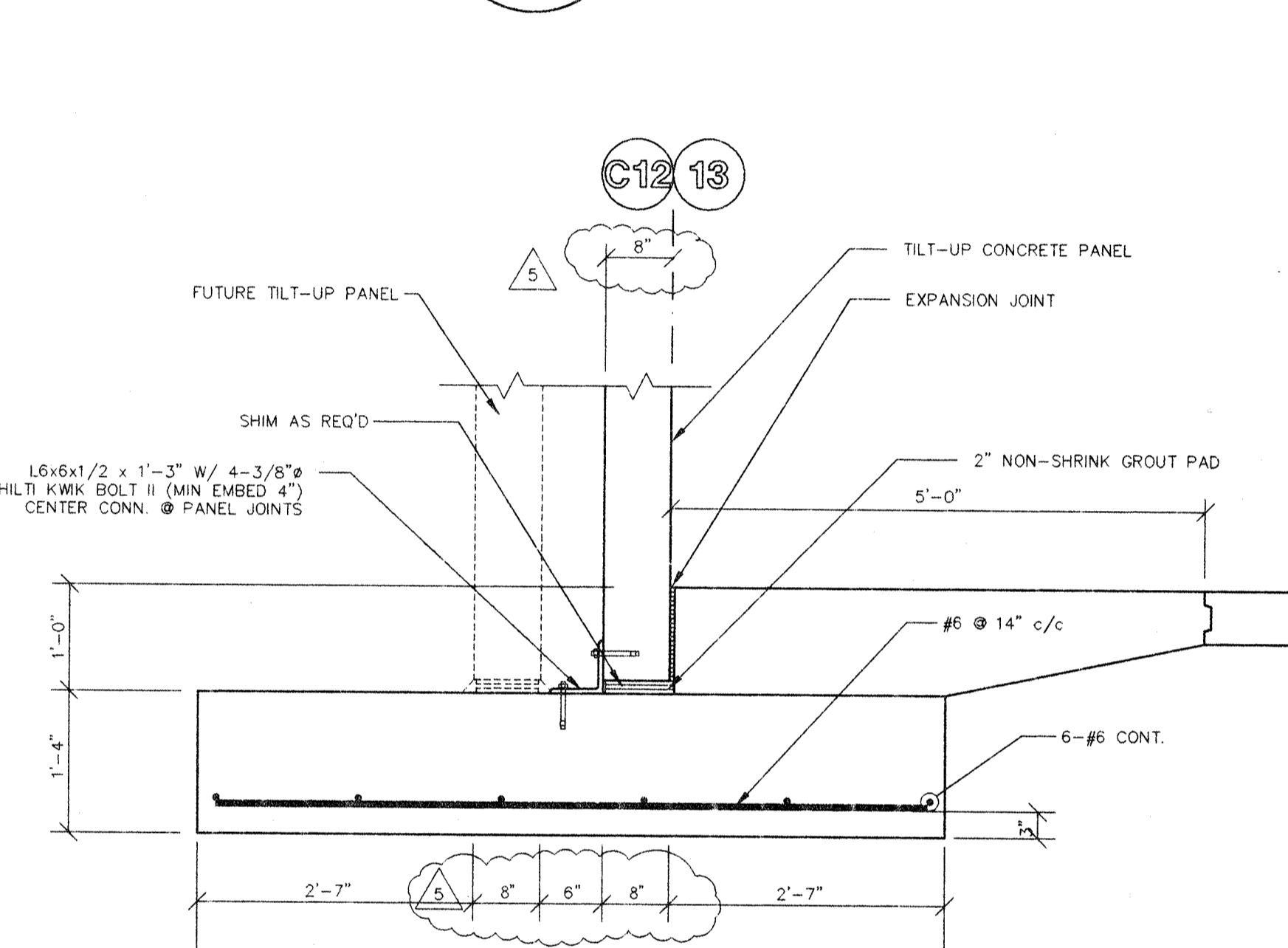
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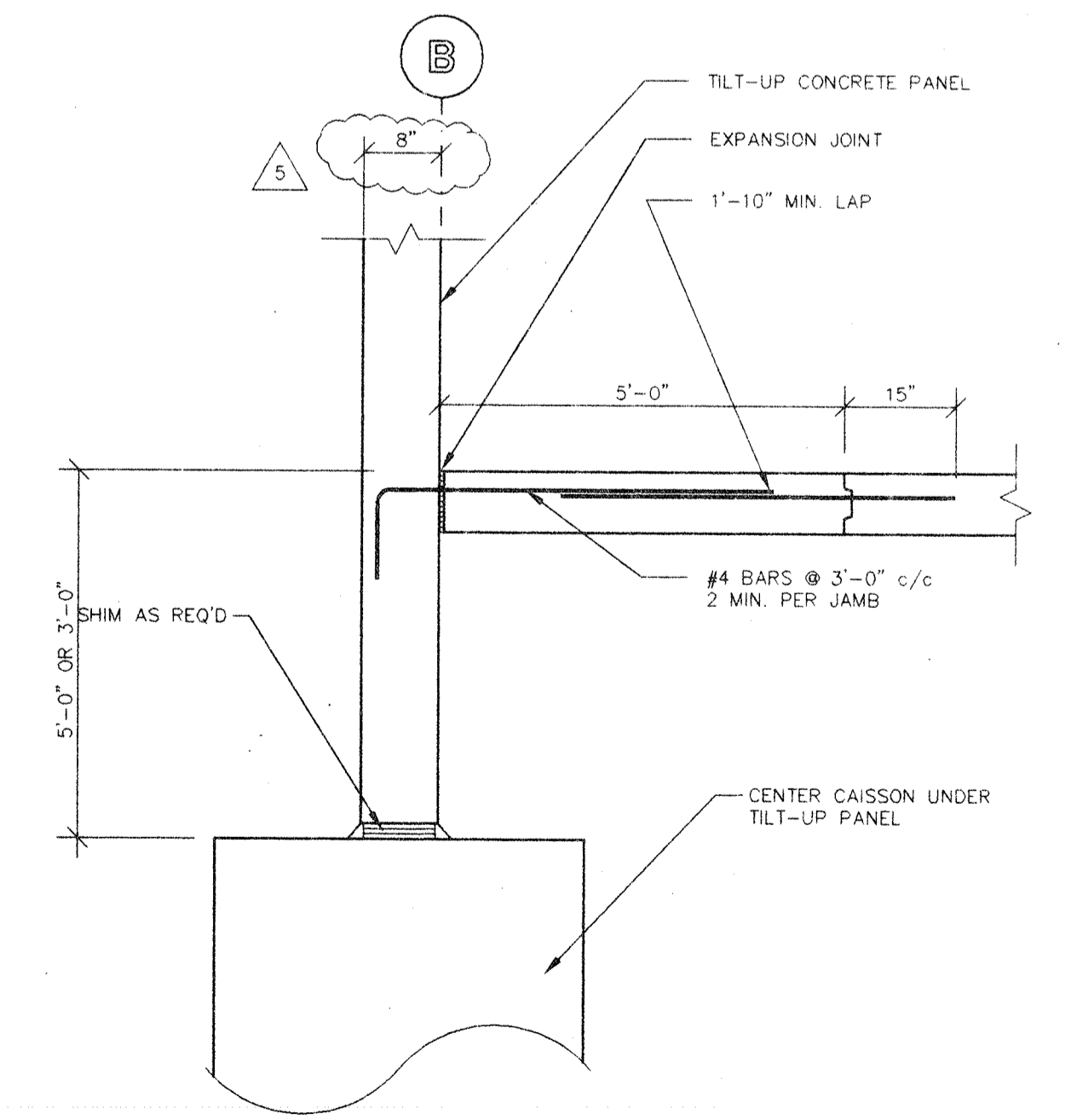
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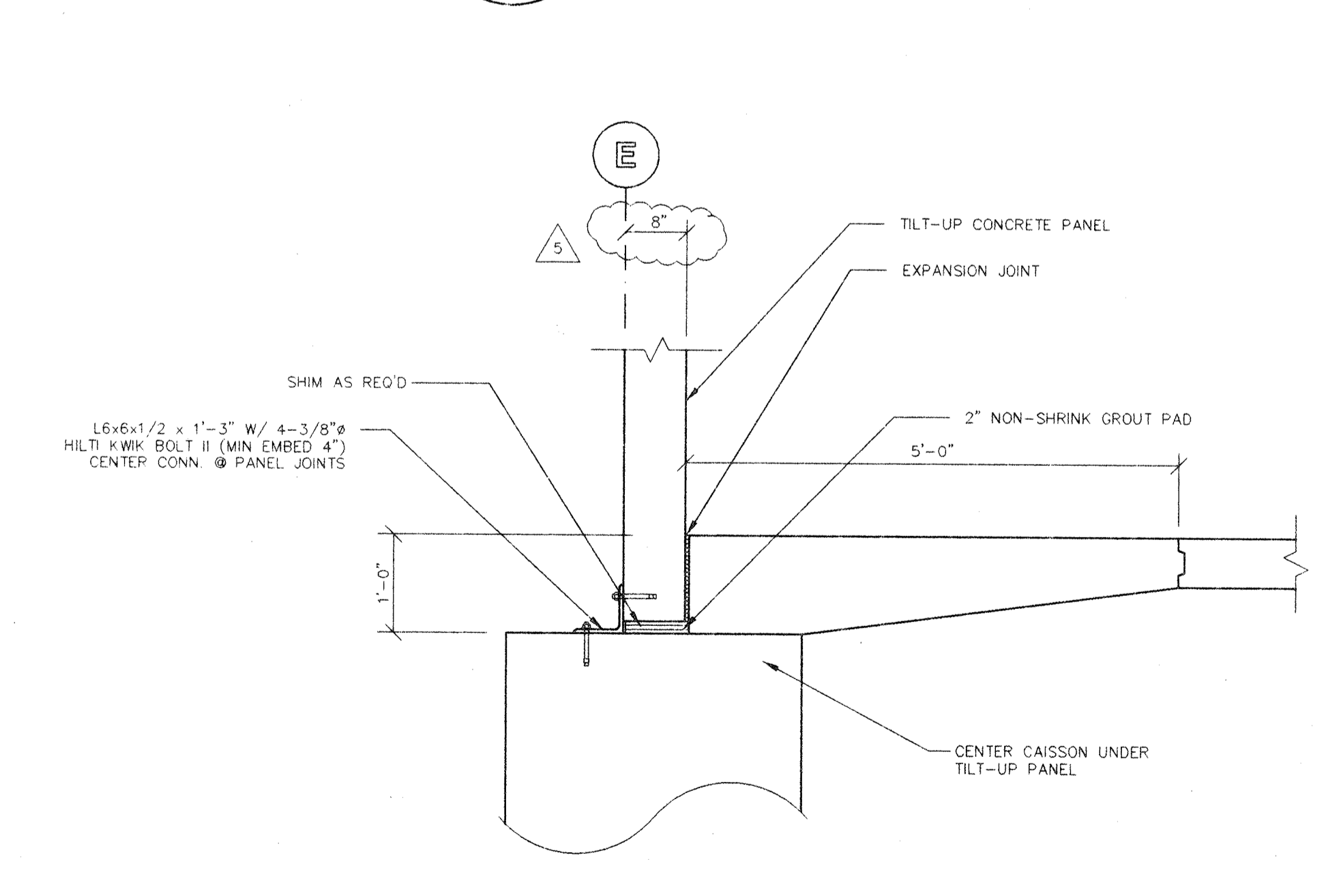
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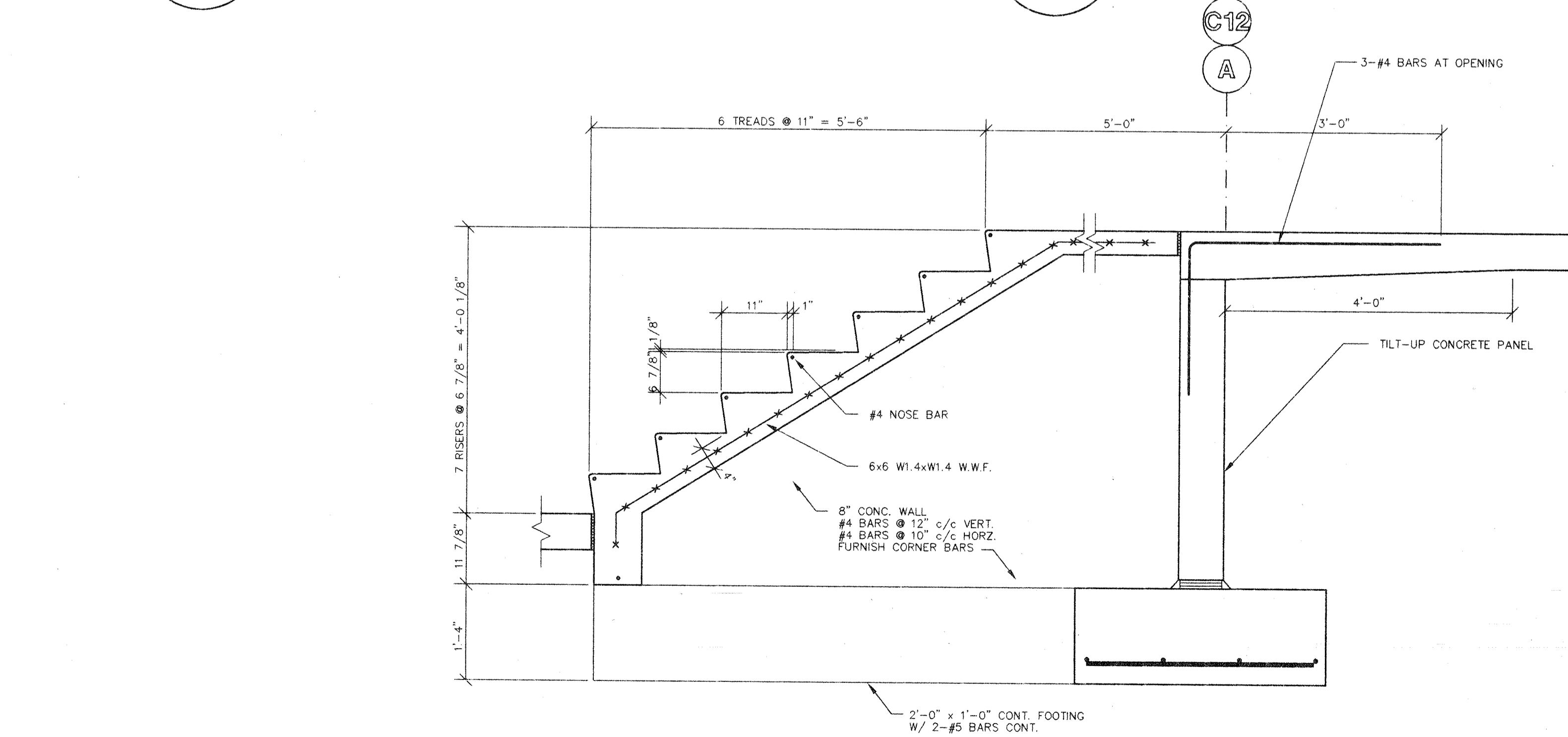
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9 SECTION  
S-3.1



10 SECTION  
S-3.1



11 SECTION  
S-3.1

DATE	DESCRIPTION
12/19/94	RELEASED FOR CONSTRUCTION
11/21/94	STRUCTURAL REVISIONS
11/15/94	STRUCTURAL REVISIONS
10/21/94	SHELL APPROVAL

**Gray**  
2500 Financial Center  
P.O. Box 277  
Glasgow, Kentucky  
[502] 281-5000

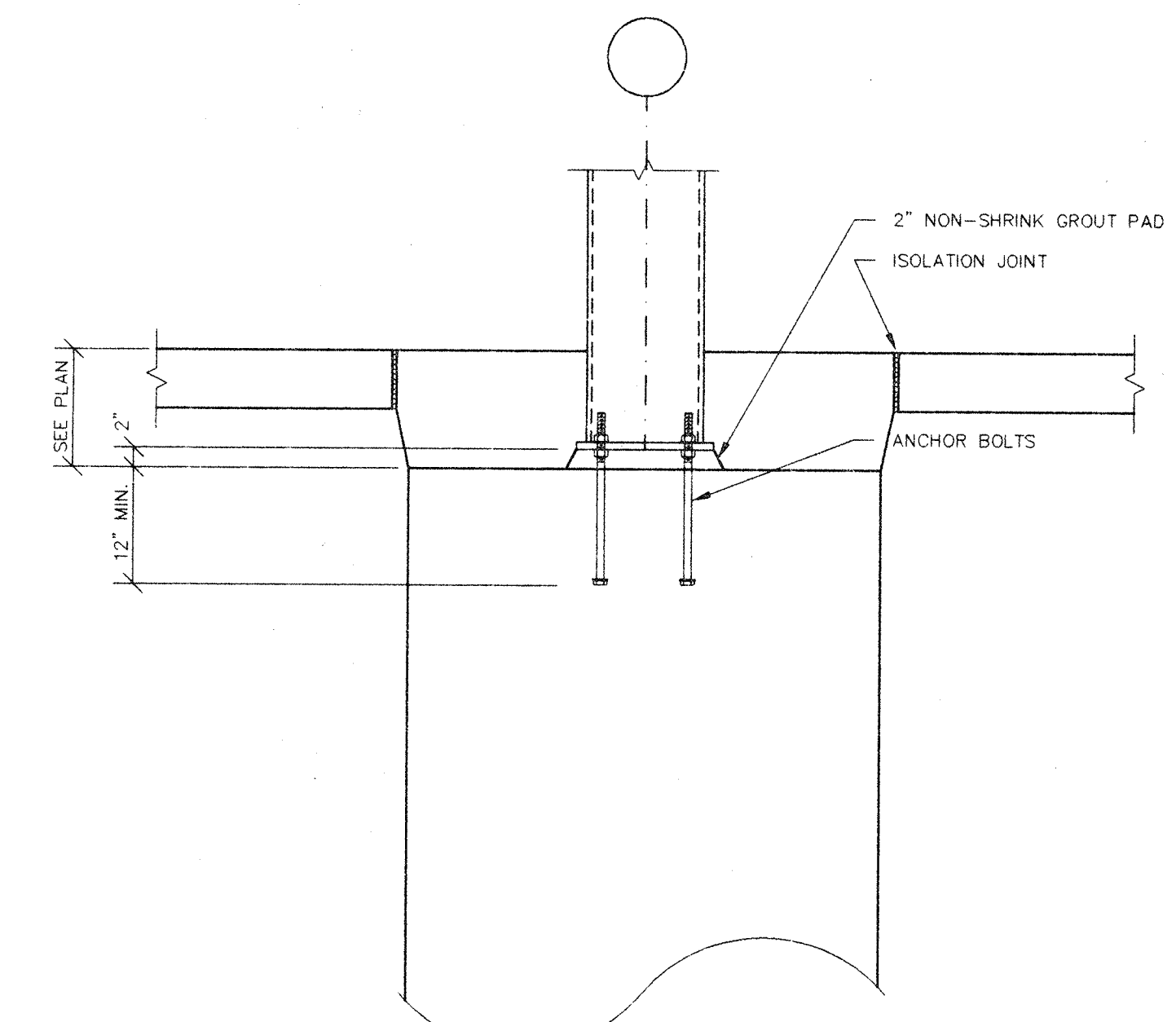
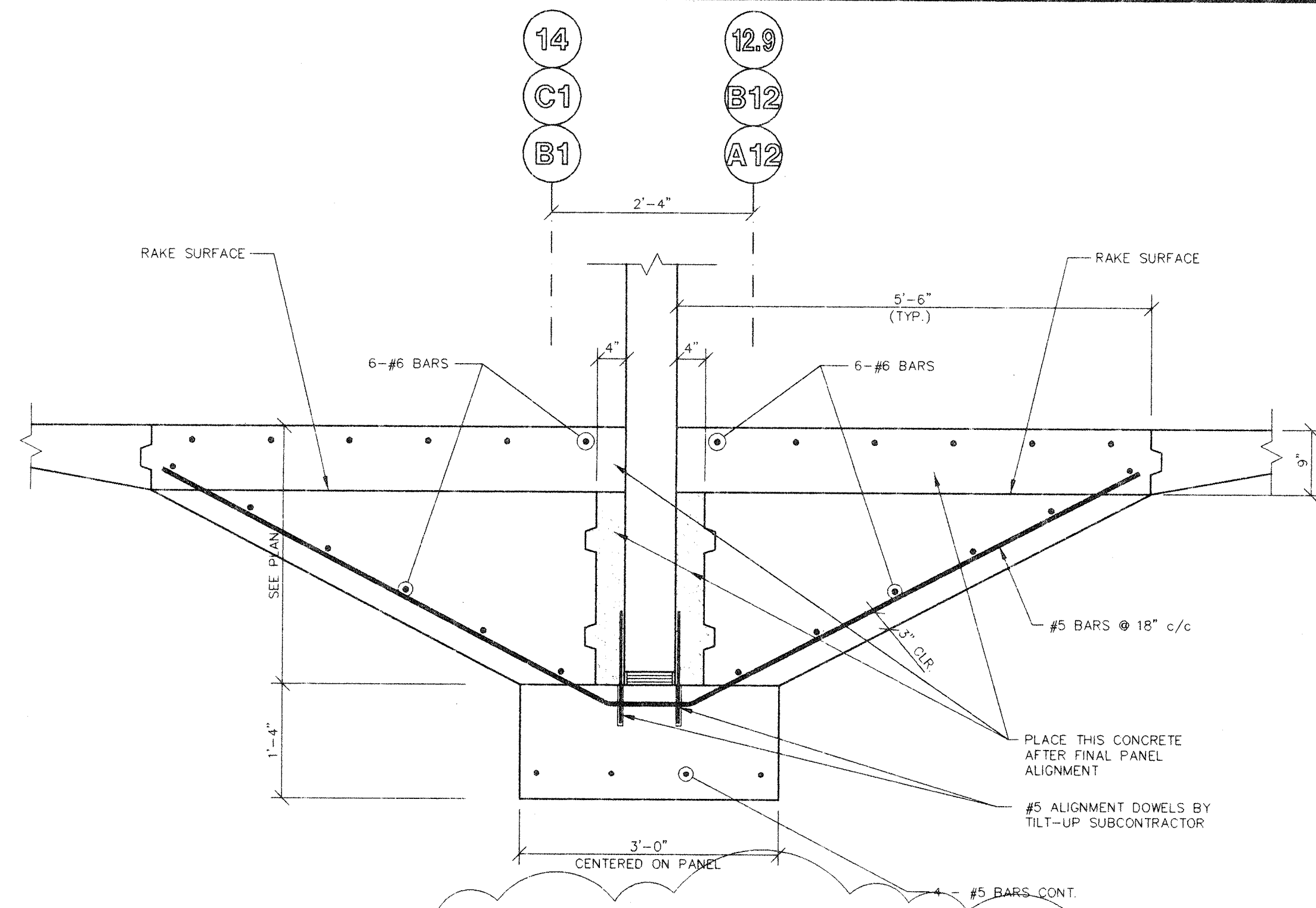
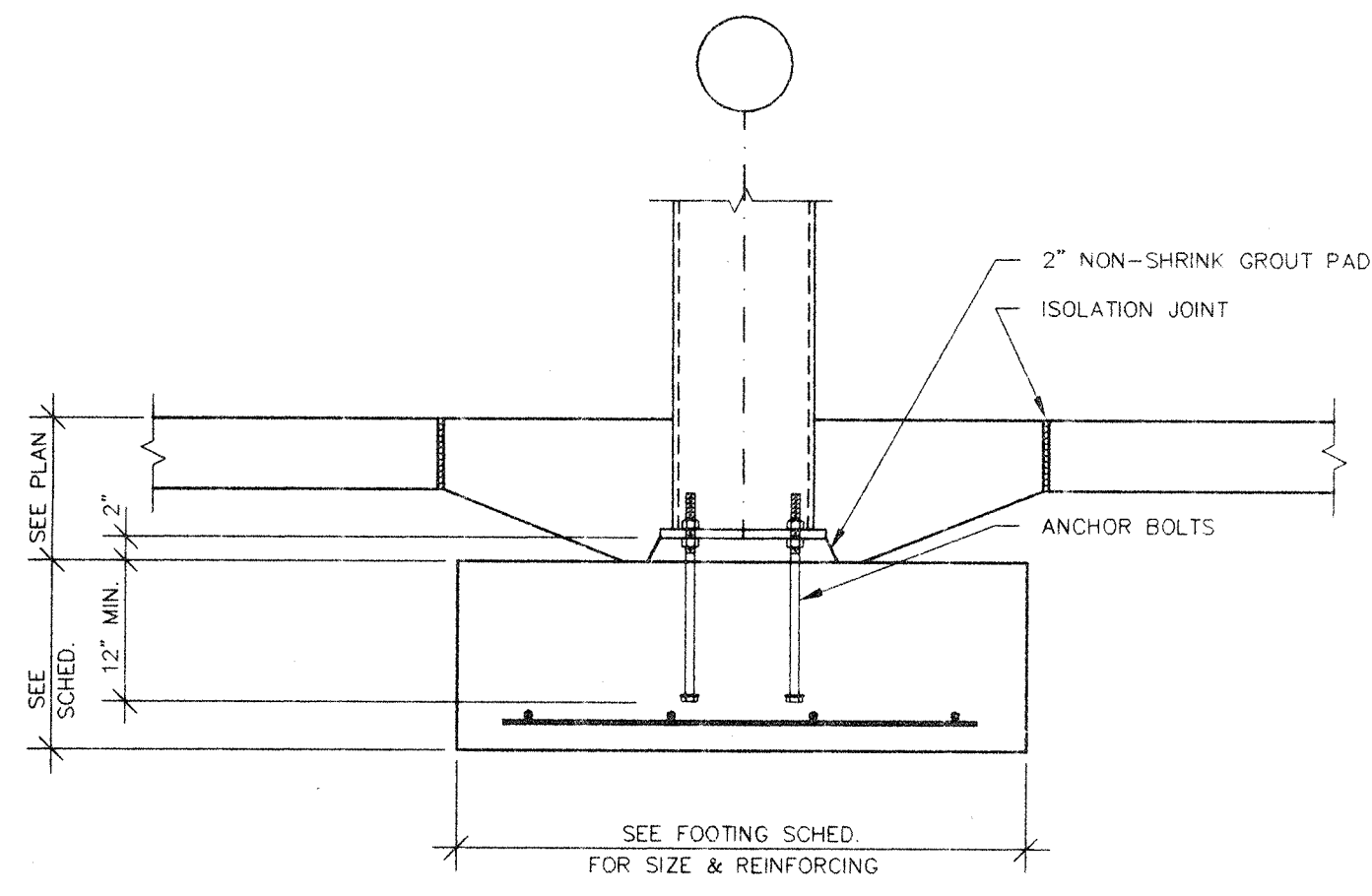
**James N. Gray Construction Co.**  
Design - Engineering - Construction

CONSTRUCTION DOCUMENT

MARTIN MILLS, INC.  
VIDALIA APPAREL DIVISION  
VIDALIA, LOUISIANA

FOUNDATION SECTIONS

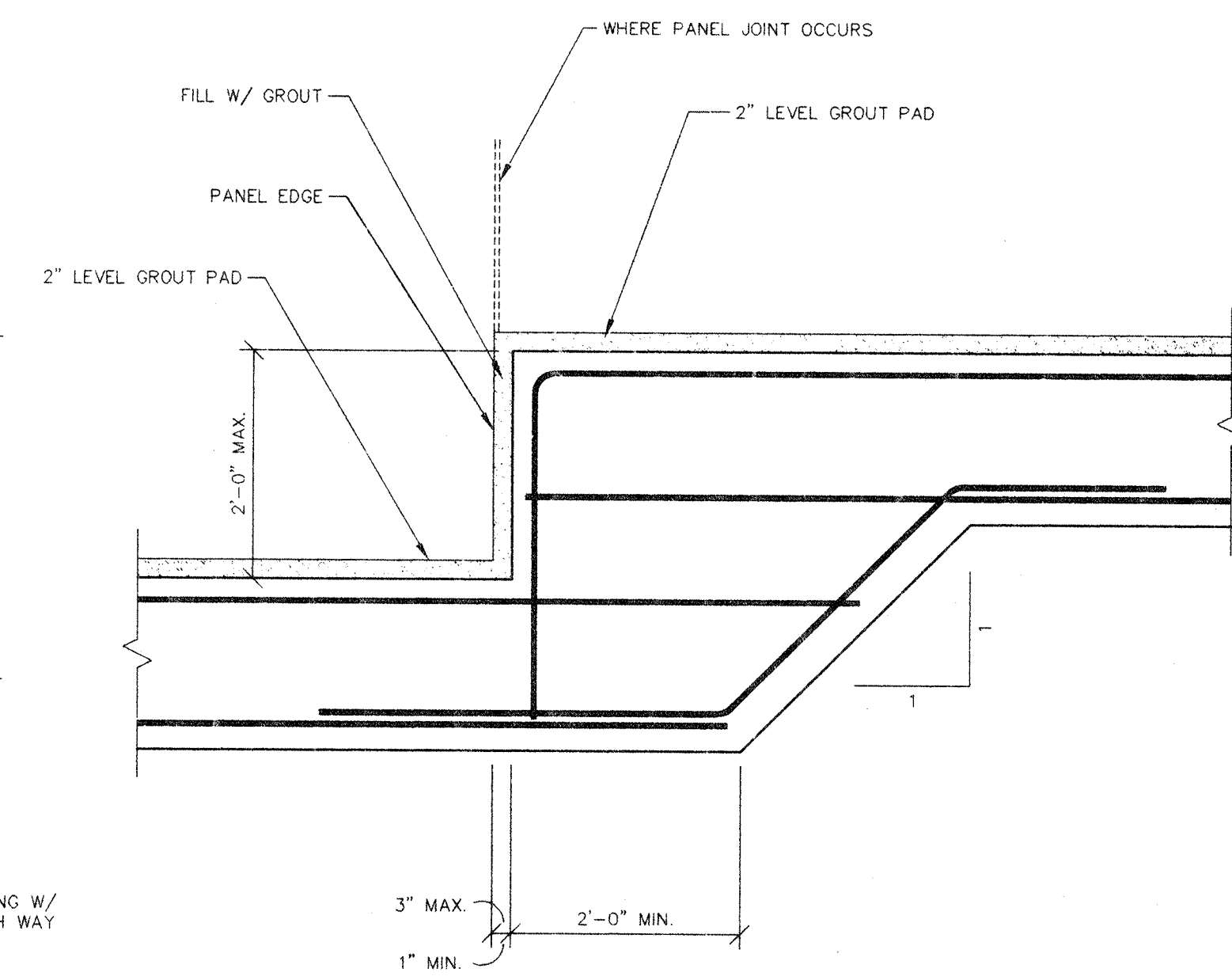
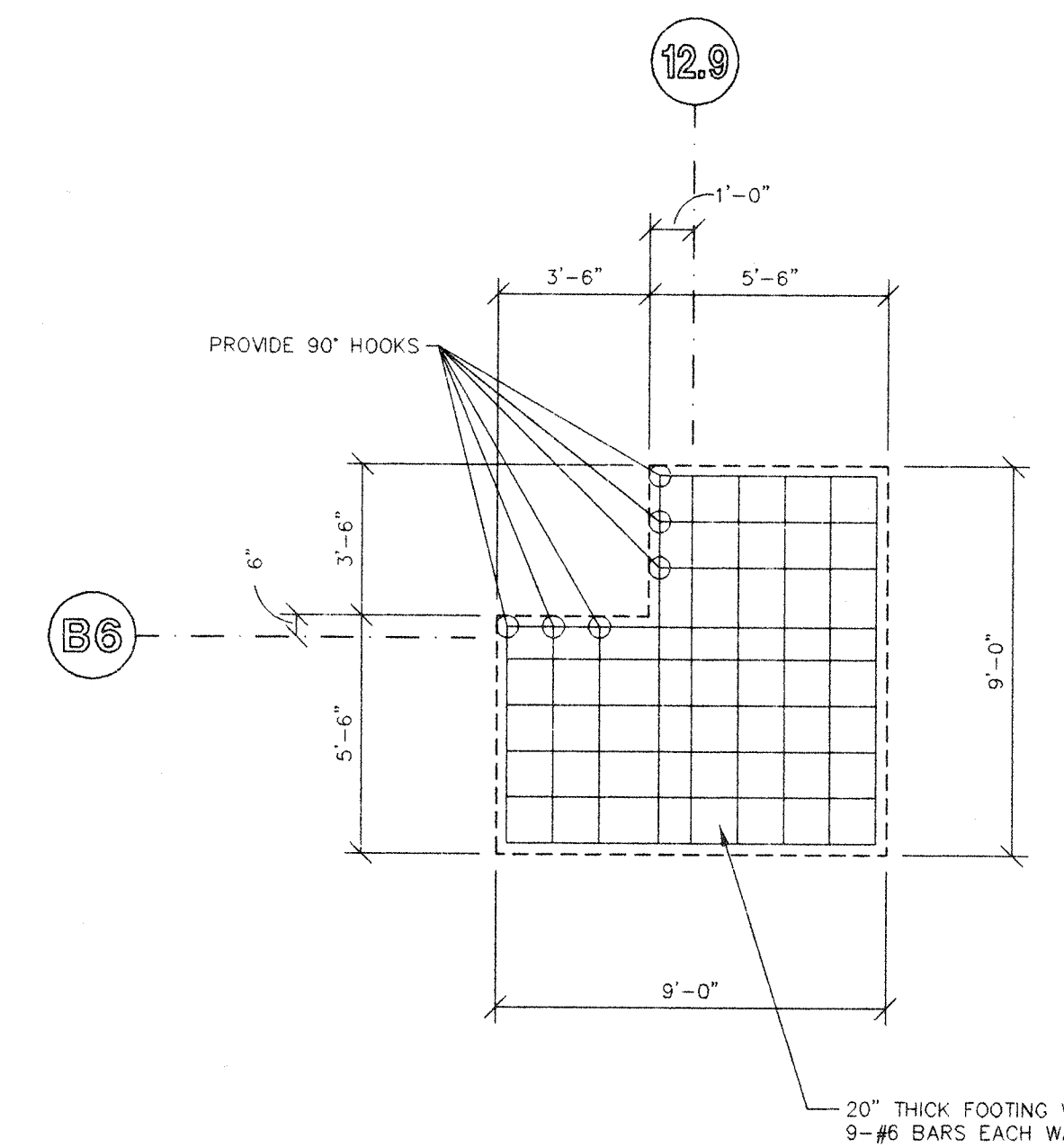
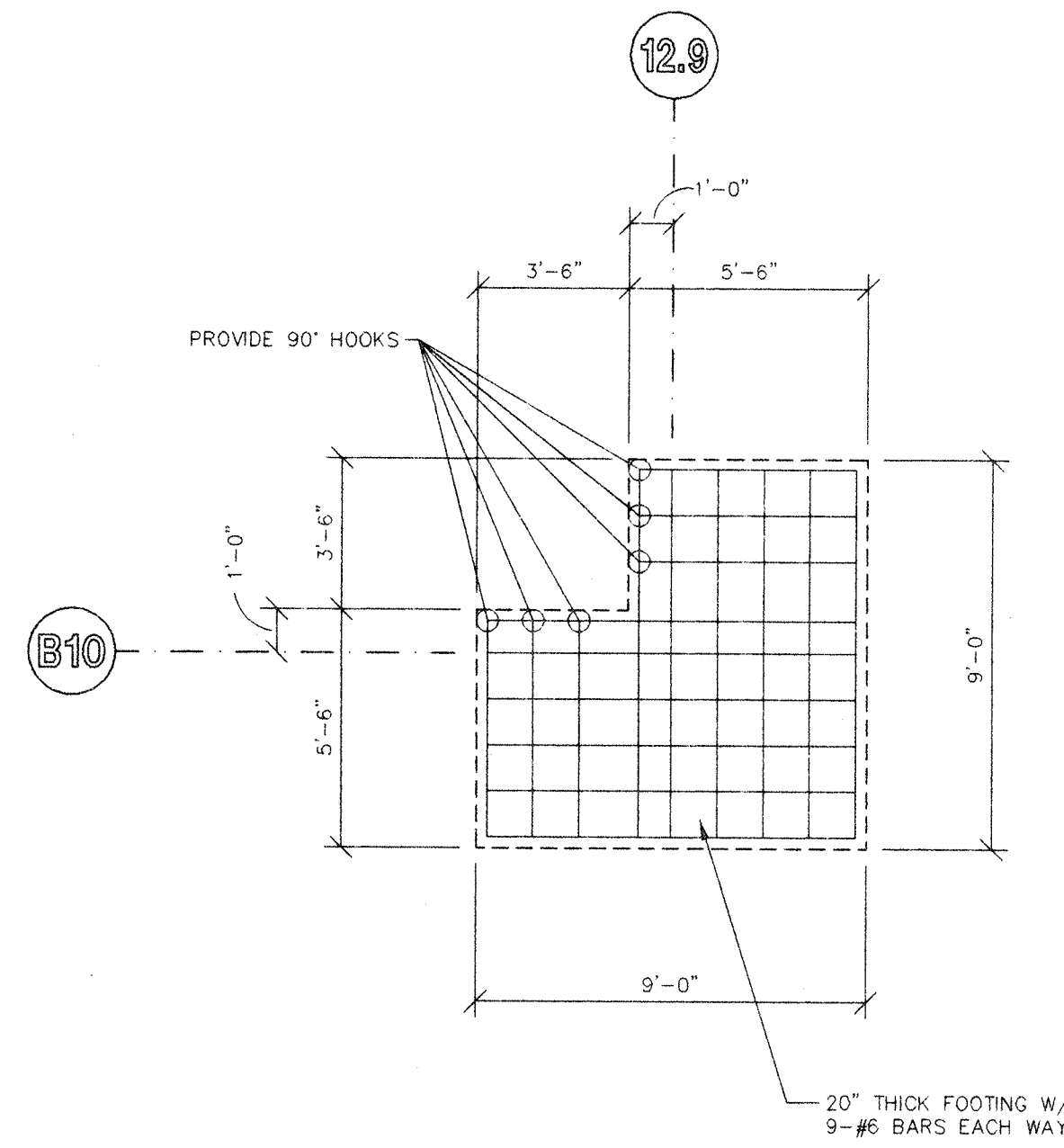
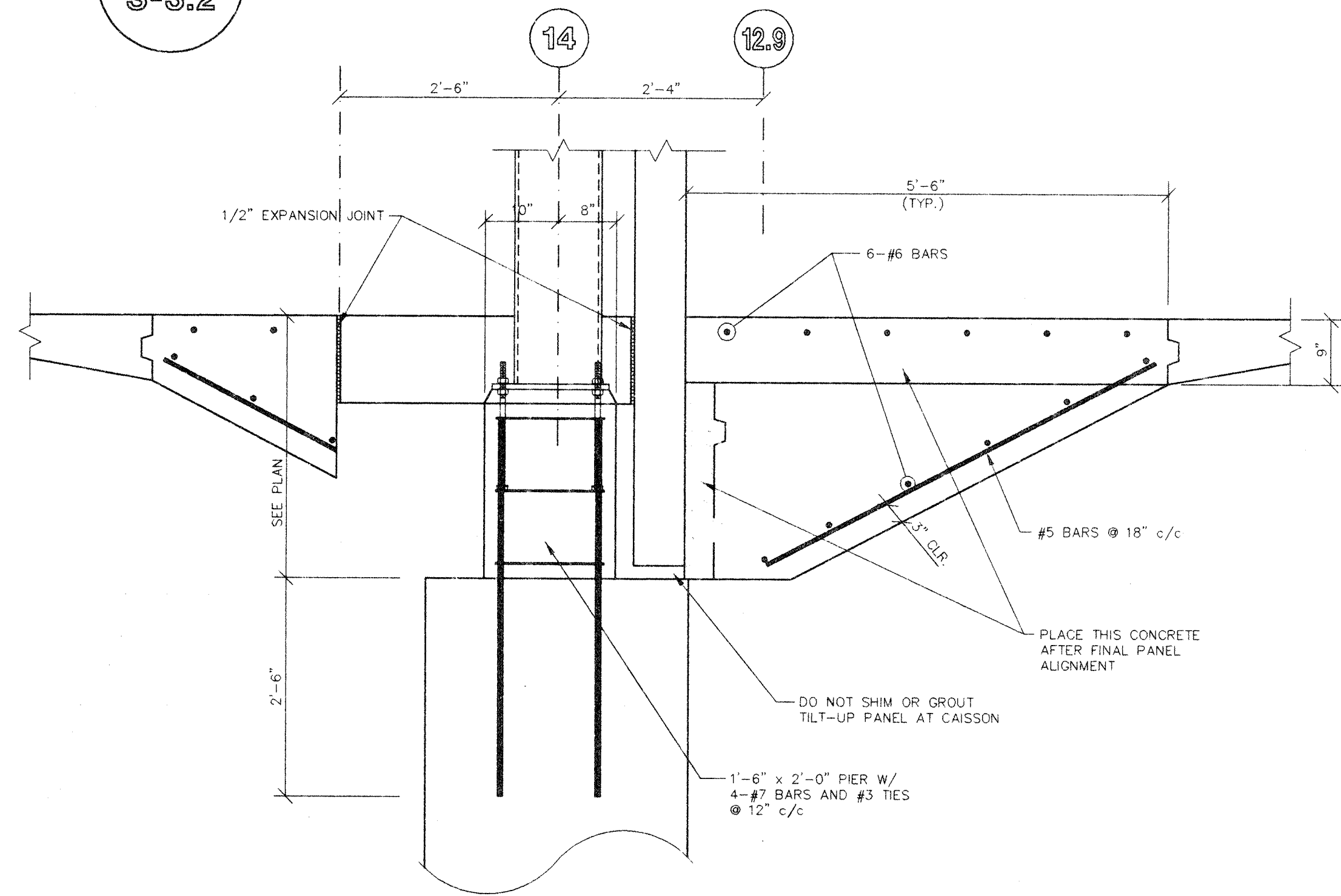
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S-3.2

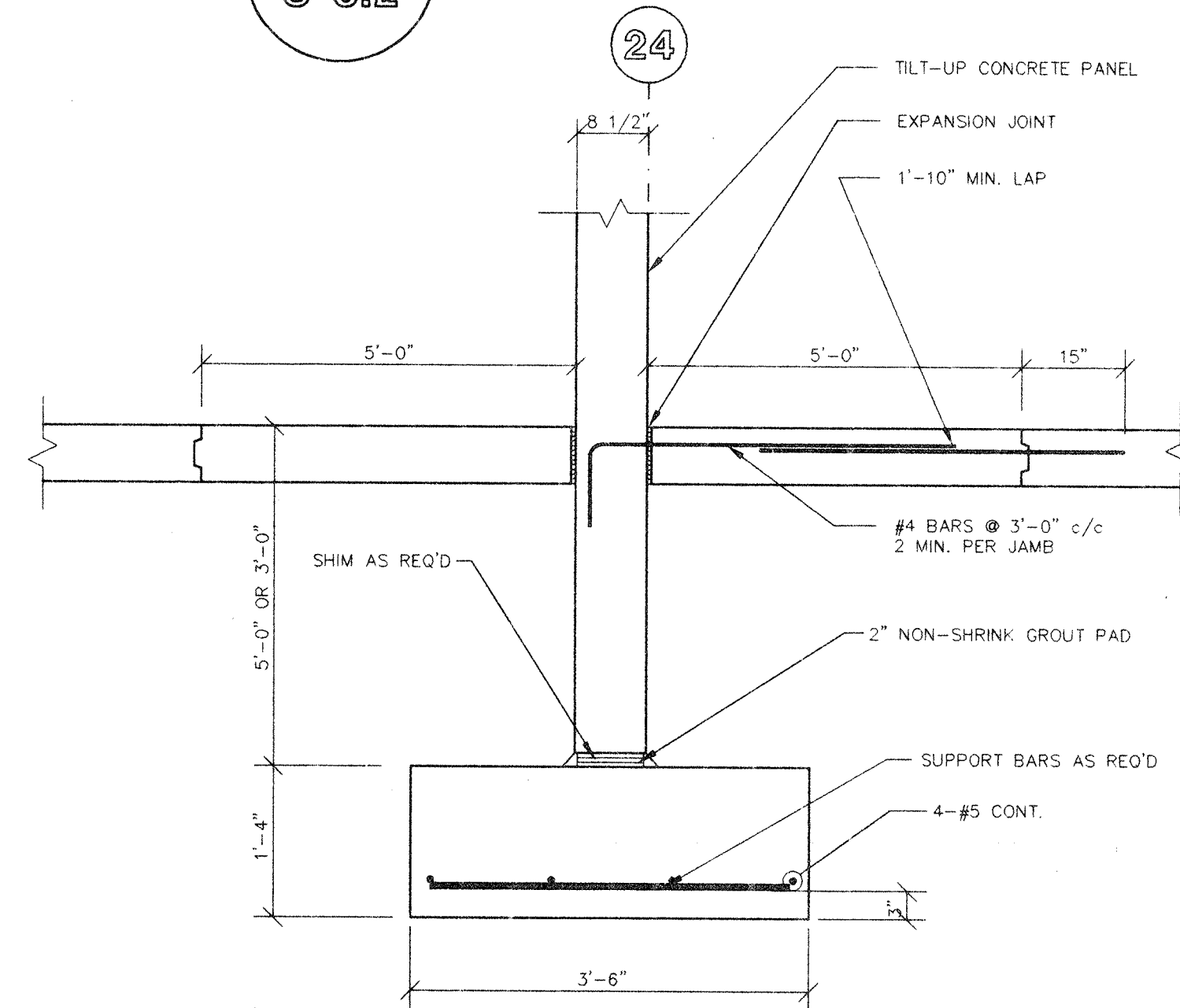
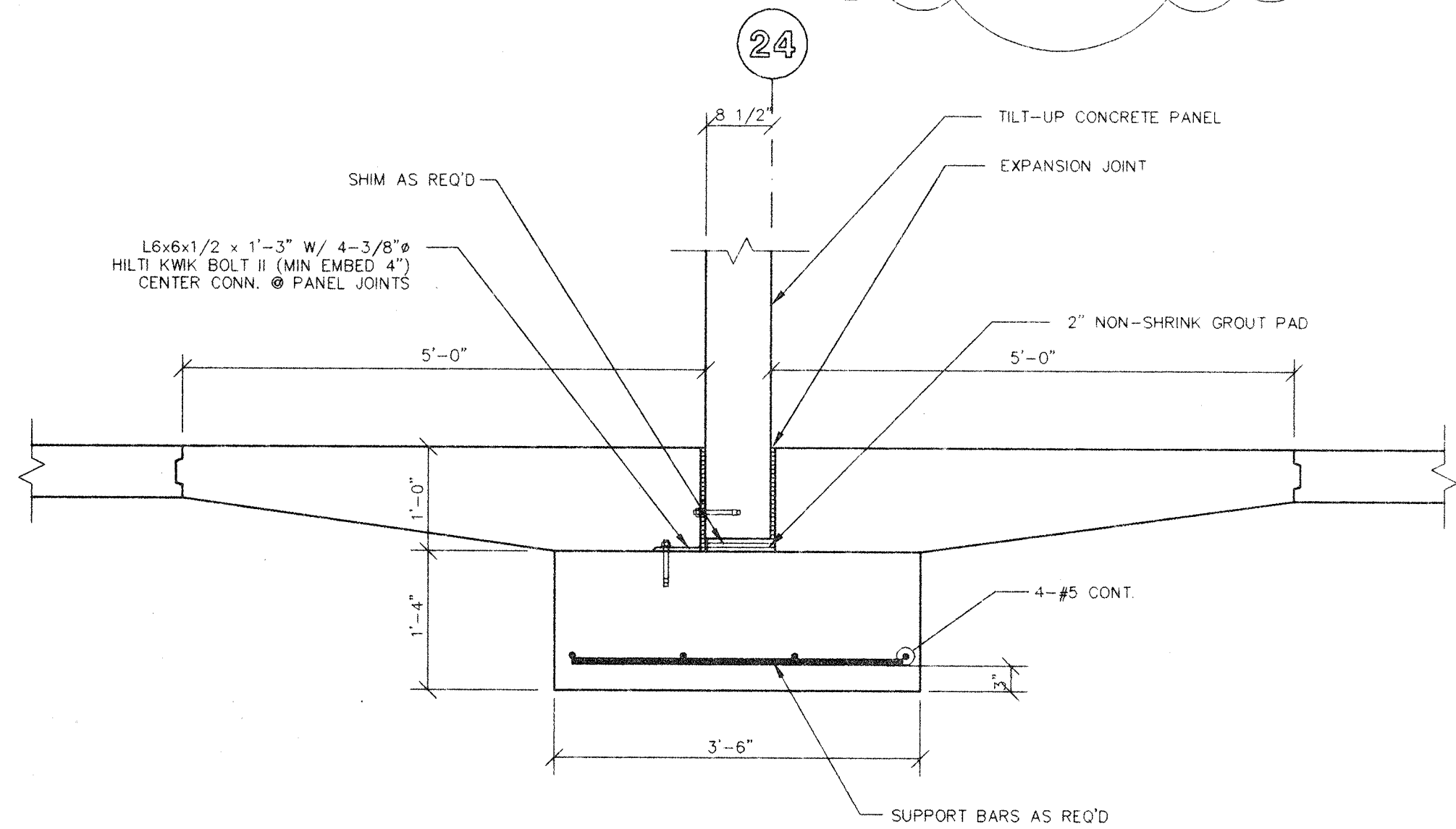
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4 SECTION  
S-3.2

5 SECTION  
S-3.2

TYPICAL FOOTING STEP DETAIL



6 SECTION  
S-3.2

7 SECTION  
S-3.2

DATE	DESCRIPTION
12/19/94	RELEASED FOR CONSTRUCTION
11/21/94	STRUCTURAL REVISIONS
11/15/94	STRUCTURAL REVISIONS
10/21/94	SHELL APPROVAL

**Gray**

2500 Financial Center  
P.O. Box 277  
Glasgow, Kentucky  
[502] 651-8891

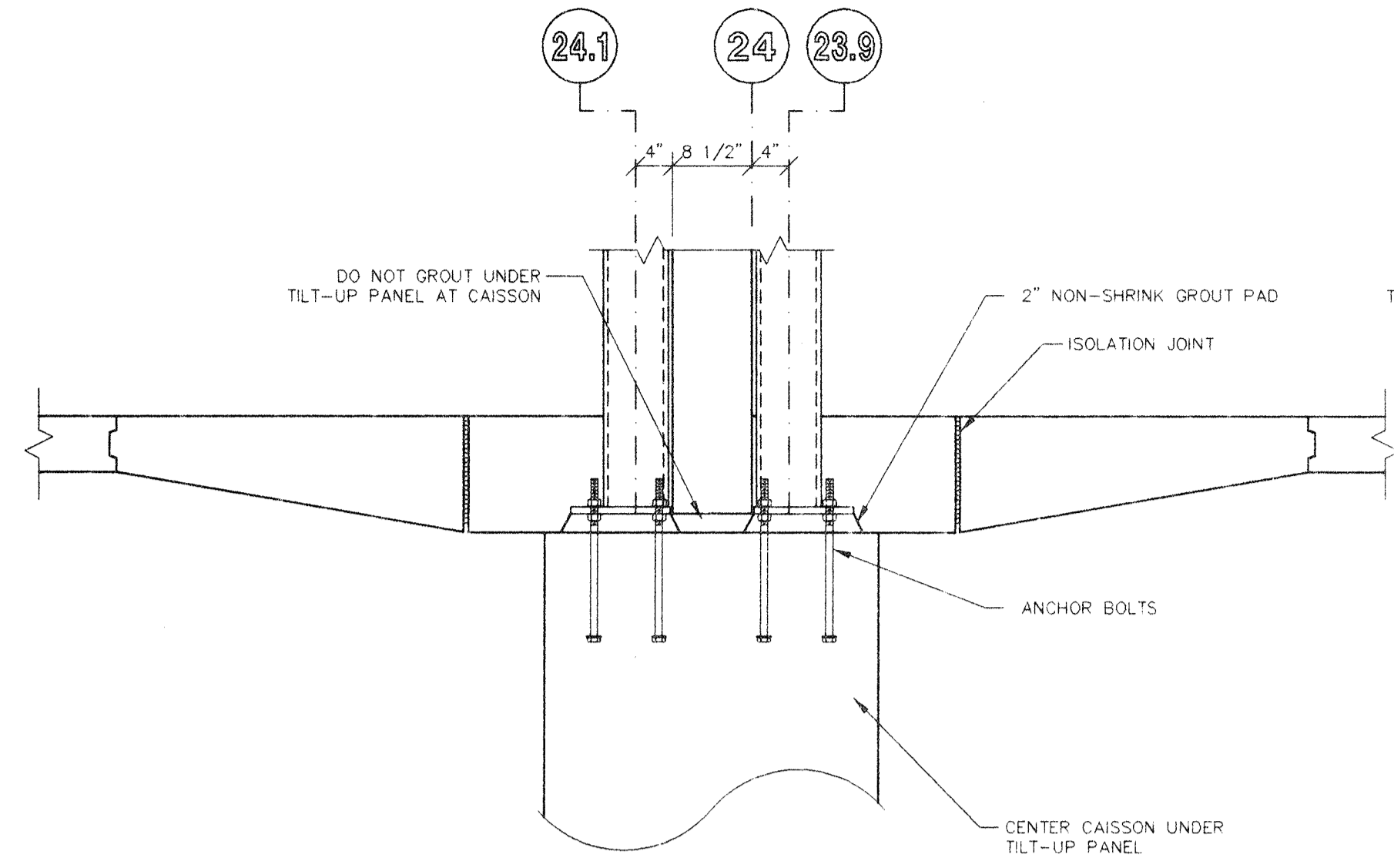
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CONSTRUCTION DOCUMENT

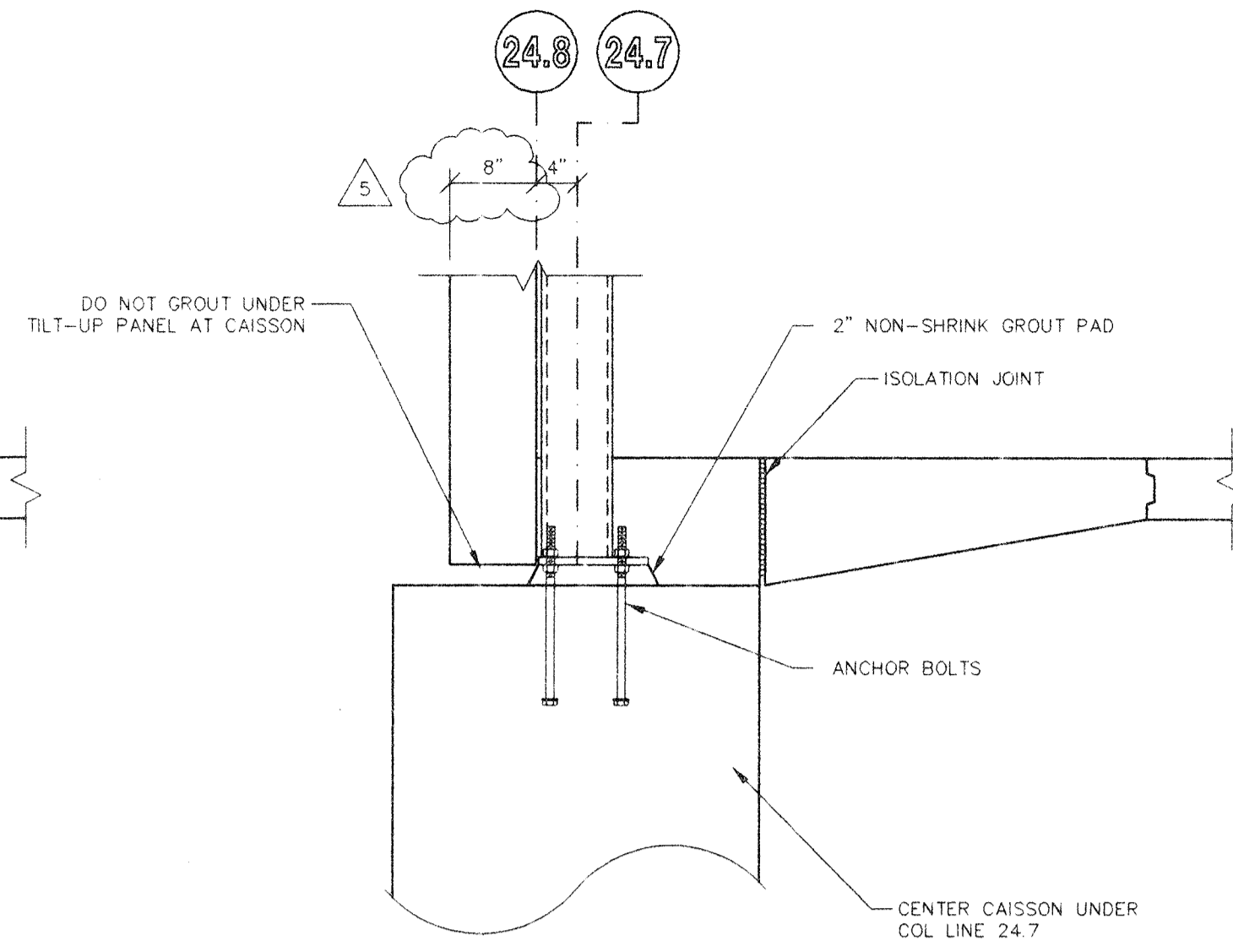
MARTIN MILLS, INC.  
VIDALIA APPAREL DIVISION  
VIDALIA, LOUISIANA

FOUNDATION SECTIONS

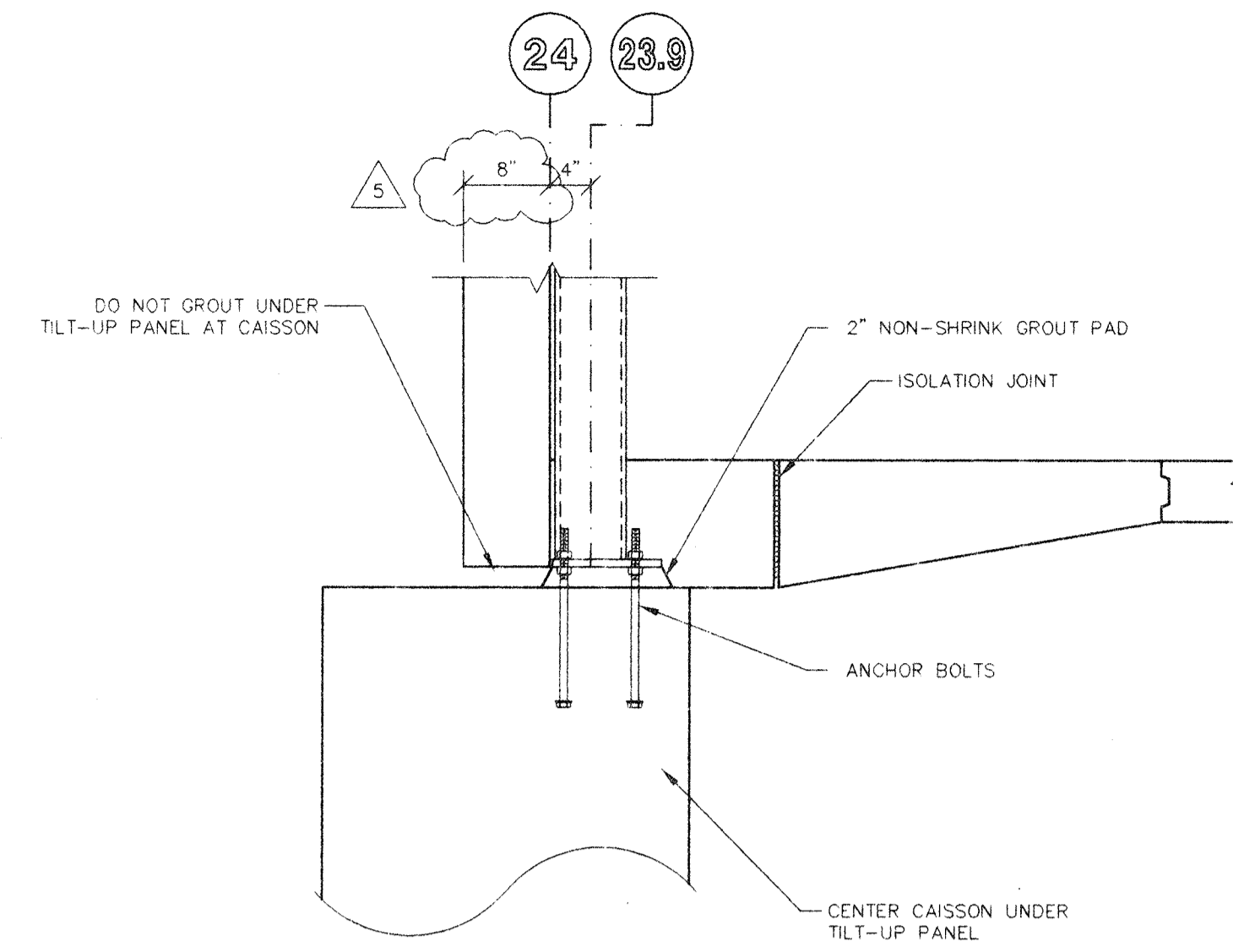
JOB NUMBER	SHEET NUMBER
95067	S-3.2



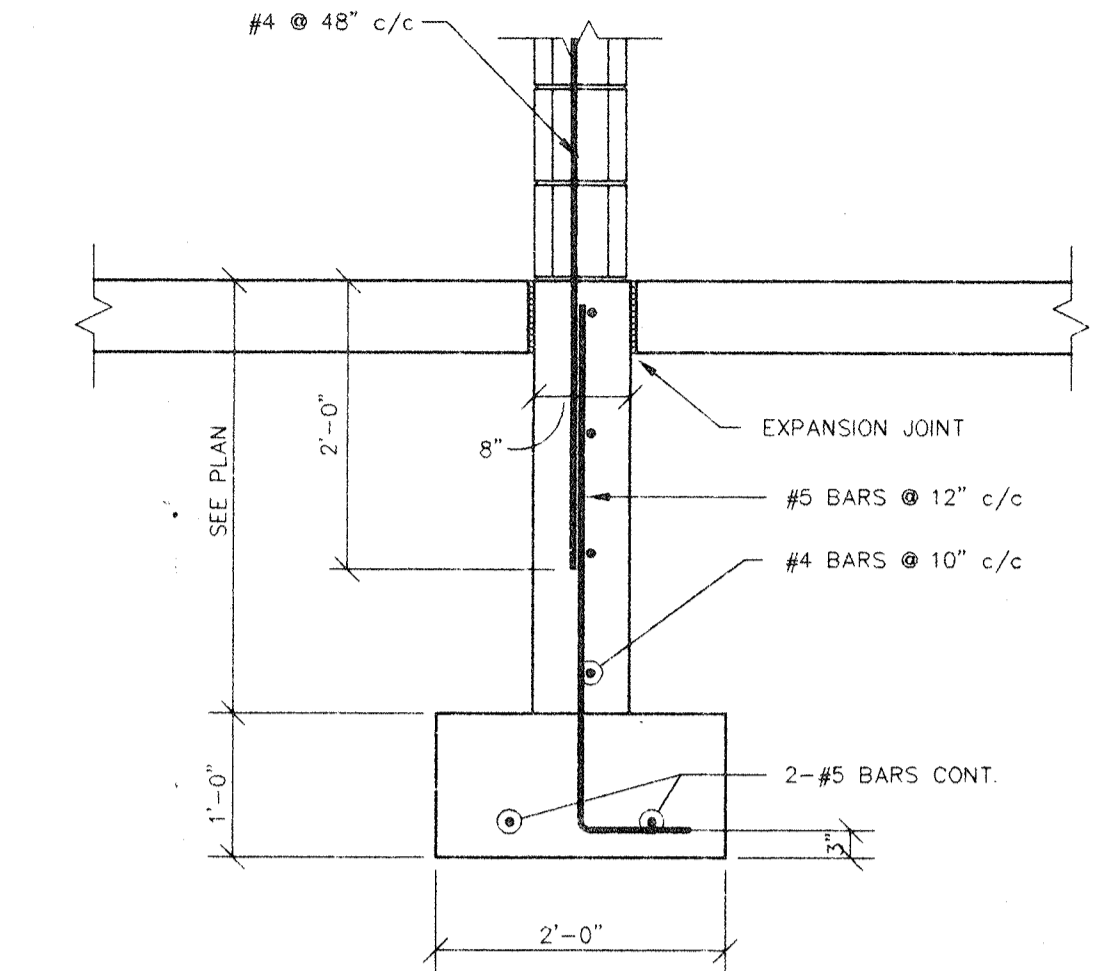
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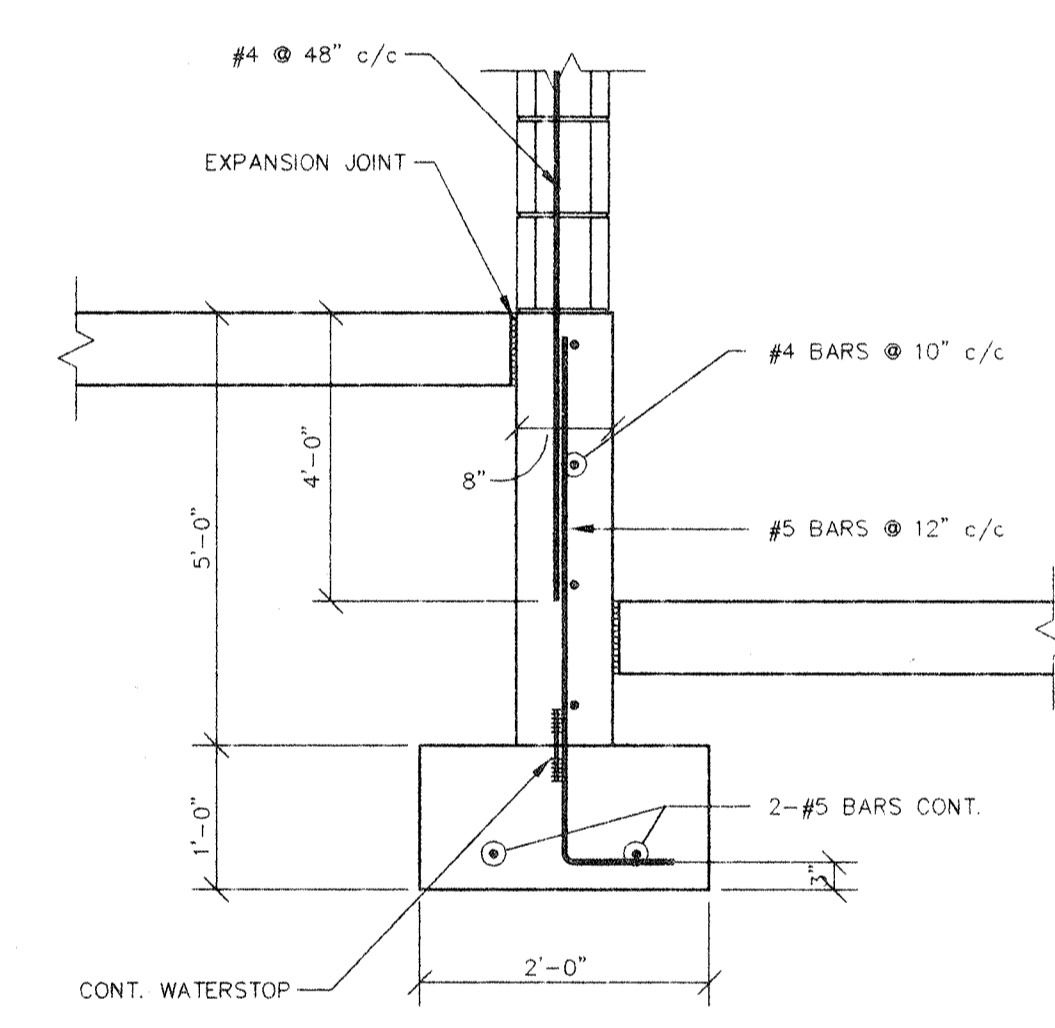
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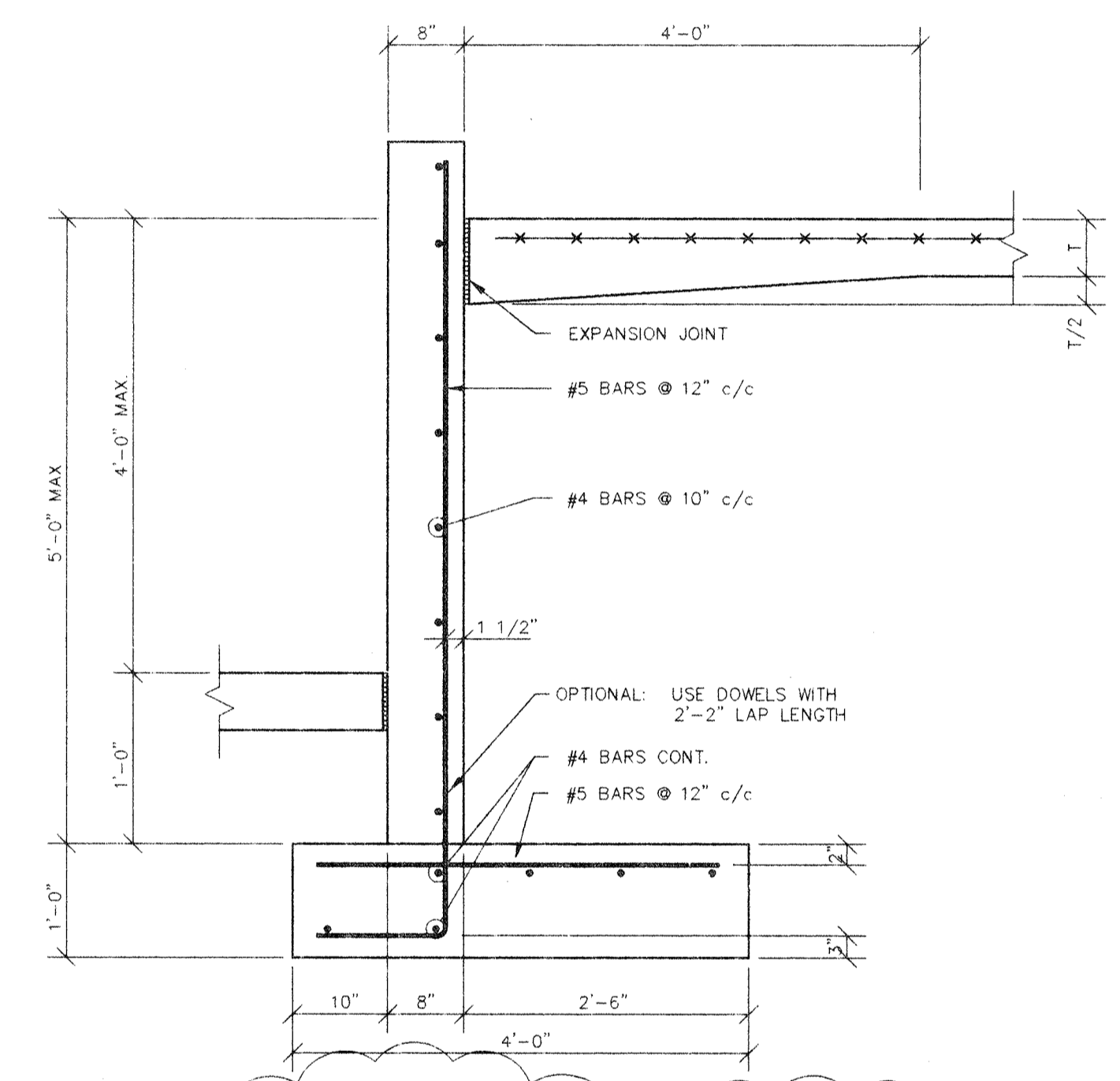
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4 SECTION  
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5 SECTION  
S-3.3



6 SECTION  
S-3.3

DATE	DESCRIPTION
12/19/94	RELEASED FOR CONSTRUCTION
11/21/94	STRUCTURAL REVISIONS

**Gray**

2500 Financial Center  
250 W. Main Street  
Lexington, Kentucky  
[606] 281-5000

**James N. Gray Construction Co.**  
Design - Engineering - Construction

CONSTRUCTION DOCUMENT

MARTIN MILLS, INC.  
VIDALIA APPAREL DIVISION  
VIDALIA, LOUISIANA

FOUNDATION SECTIONS

JOB NUMBER 95067	SHEET NUMBER S-3.3
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