



Port of Terrebonne - West Bank

Exhibit 26 – Geotechnical Exploration Report

GEOTECHNICAL EXPLORATION REPORT

**PORT OF TERREBONNE
LED SITE CERTIFICATION
ROME WOODWARD DR.
HOUMA, LA**

FOR

**DELTA COAST CONSULTANTS, LLC
HOUMA, LA**

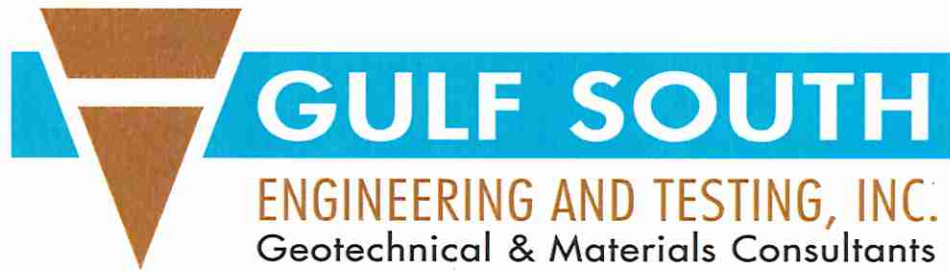
GULF SOUTH ENGINEERING AND TESTING FILE NO. 24-004

March 6, 2024



15 Veterans Blvd. Kenner, LA 70062

PN: 504.305.4401 FN: 504.305.4408 E-mail: info@gulfsoutheng.com



15 Veterans Memorial Boulevard, Kenner, LA 70062
PN: 504-305-4401 FN: 504-305-4408 E-mail: info@gulfsoutheng.com

March 6, 2024

Delta Coast Consultants, LLC
4924 Highway 311
Houma, LA 70360

Attn: Amber Plessala, P.E.
Email: amber.plessala@deltacoastllc.com

Re: Geotechnical Exploration Report
Port of Terrebonne
LED Site Certification
Rome Woodard Dr.
Houma, LA
Gulf South Engineering & Testing File No. 23-047

Dear Amber,

Please find attached our geotechnical exploration report that was completed for the referenced project. We appreciate the opportunity to serve your geotechnical needs. Please contact us should you have any questions.

Sincerely,

GULF SOUTH ENGINEERING AND TESTING, INC.

CHAD M. POCHÉ, P.E.
Executive Vice President

BRYSON S. BEARD, E.I.
Associate Geotechnical Engineer

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FIGURES – No. 1 Boring Plan
 No. 2 Open, Steel, Pipe Pile (OSP) Capacities
 No. 3 Square, Pre-Cast, Concrete Pile (SPC) Capacities
 No. 4 Group Effect

APPENDIX – Boring Logs

GEOTECHNICAL EXPLORATION REPORT

**PORT OF TERREBONNE
LED SITE CERTIFICATION
ROME WOODWARD DR.
HOUMA, LA**

GULF SOUTH ENGINEERING AND TESTING FILE NO. 24-004

1.0 INTRODUCTION & LIMITATIONS

This report contains the results of a preliminary geotechnical exploration made at the subject site. The investigation is preliminary as specific details for the project are not known at the time of the investigation. Further investigation of the site may be warranted once details become finalized. Instructions to proceed with the exploration were received from Delta Coast Consultants, LLC (Client) via our approved proposal dated November 6, 2023.

The study included drilling soil test borings and the performance of soil mechanics laboratory tests to evaluate the soil's physical characteristics. Engineering analyses were made and based on the field and laboratory test data to develop recommendations for the project.

The analyses and recommendations presented in this report are based on the provided project information and the results of the exploration. While it is not likely that conditions will differ significantly from those observed during the field exploration it is always possible that variations can occur away from the borehole location(s).

If it becomes apparent during construction that subsurface conditions differing significantly from those observed in our boring(s) are encountered, Gulf South should be notified. Also, should the nature of the project change or should any of the stated assumptions be inaccurate, the recommendations provided in this report should be re-evaluated.

This report has been prepared for the exclusive use of our Client. The recommendations provided in this report are site specific and are not intended for use at any other site or for any other project. This report provides

recommendations for design and construction and should not be used as construction specifications.

Gulf South considers the materials testing and onsite inspection during construction an extension of our geotechnical exploration and a key component to ensuring the recommendations provided in this report are followed. For this type of project, these services may consist of earthwork testing and monitoring, pile/shaft inspection and monitoring, vibration monitoring, concrete testing and inspection, and steel inspection. Gulf South should be retained to provide the construction inspection services for this project.

2.0 SOIL BORINGS

Two (2) undisturbed soil borings were drilled to depths of 50 feet (Boring B-1) and 30 feet (Boring B-2) below the ground surface on February 5, 2024. The borings were performed using an ATV mounted drilling rig. The approximate soil boring locations are shown on Figure No. 1.

Undisturbed sampling was performed continuously or on approximate 5-foot centers in all cohesive or semi-cohesive materials with a three-inch diameter thin wall tube sampler. The samples were extruded in the field, representative portions of each sample were trimmed and placed in moisture proof containers, the samples were properly labeled, and secured for transport to the laboratory.

When cohesionless material was encountered or when soils could not be adequately sampled by undisturbed methods, the Standard Penetration Test was performed. This test consists of driving a two-inch diameter split spoon sampler a total of approximately 18 inches with a 140 lb. hammer falling 30 inches. The number of blows required to drive the sampler per 6-inch increment is recorded and gives an indication of the density of the material. The blows per foot shown on the boring log are the total of the blow counts for the final 12 inches of penetration.

3.0 LABORATORY TESTING

Soil mechanics laboratory tests were performed on samples obtained from the borings. The testing consisted of natural moisture content, unit weight, Atterberg limits, and unconfined or tri-axial compression strength testing. The results of the laboratory tests are shown on the soil boring logs provided in the Appendix of this report.

4.0 SUBSOIL CONDITIONS

4.1 Subsoil Description

Reference to the borings shows medium stiff clay from the ground surface to the approximate 2 foot depth below the ground surface. Below, interbedded layers of very soft to medium stiff organic clay and peat follow to the approximate 18 to 23 foot depths below the ground surface. Very soft to soft clay is present to the deepest boring's termination at the approximate 50 foot depth below ground surface.

4.2 Groundwater

At the time of making the borings, the groundwater was not encountered before wash drilling methods were implored at the approximate 10 foot depth. These observations were made during a short period of time and groundwater may not have become fully realized at the time of observation. Groundwater can fluctuate with seasonal precipitation, drainage, and prolonged drought. If the depth to groundwater is important to construction, it should be measured at that time.

5.0 FURNISHED INFORMATION AND FOUNDATION RECOMMENDATIONS

The purpose of this exploration is to provide a preliminary exploration for a Louisiana Economic Development (LED) Site Certification. We understand the aim of the exploration is to characterize the subsoils of the site and to provide both shallow and deep foundation recommendations. The recommendations provided are generalized for this site and we recommend further exploration and testing once site plans are finalized.

The near surface site conditions are not conducive for support of structures using shallow foundations in their current state unless the structures are very lightly loaded and can tolerate settlement. Ground improvement of the site would be needed to support heavier or settlement sensitive structures using shallow foundations. If shallow foundations are selected, footings should be placed to bear at least 2 feet below the ground surface within firm in-place soils or compacted select fill. Alternatively, should the values provided in this report for bearing and settlement not be tolerable, deep foundations should be used for support.

Structural analyses and the structural adequacy of the foundations are outside our scope of work for the project. Utilities to and from the structures should be attached to the slabs using suitable hangers and flexible connections.

Preliminary laboratory test results indicate the near surface soils primarily consist of fat and organics clays that have minimal to moderate shrink/swell potential. Care should be taken during and after construction to limit activities that could affect moisture within the soils below and around the foundations. By precluding surface waters from saturating the soils, the resulting volumetric movements will be minimized. In this regard, good surface drainage should be assured with positive collection and runoff of these waters.

6.0 SHALLOW FOUNDATIONS FOR LIGHTLY LOADED STRUCTURES

6.1 Net Allowable Soil Bearing Capacity

We estimate a net allowable soil bearing capacity of 250 lbs. per sq. ft. (psf) is available for design of shallow strip or square footings, respectively for lightly loaded structures. This allowable soil bearing capacity assumes the footings are bearing at least 2 feet deep and are seated within firm in-place soils or compacted select fill.

Foundation excavations should be thoroughly inspected to assure that the footings are seated in firm and well-drained soil. The allowable soil bearing capacities contain a factor of safety of at least 3.0 against failure but do not preclude settlements, as will be discussed.

6.2 Estimated Settlement

Footings. Settlement analyses were made using applied pressures equal to 100% of the allowable soil bearing values. Long-term settlement of square footings no larger than 6 feet in width and strip footings no wider than 3 feet in width is estimated to be 1 inch or less. Settlement will increase with the size of the footing and/or loading and if larger footings are needed for support, revised settlement analyses should be made.

In view of the magnitude of the estimated settlement and to bridge any undetected soft or loose areas, good rigidity should be assured in the foundations to minimize the effects of differential settlements. This may be accomplished by using a post tensioned slab, a ribbed or waffle type slab, etc.

Adequate steel reinforcement should be designed and included within the foundations. If the estimated settlements for shallow footings are considered prohibitive, deep foundations should be used for support.

6.3 Site Preparation and Fill Materials

Prior to construction, the foundation areas should be stripped of all vegetation, debris, soft or loose surface soils, deleterious materials, etc., and should be well drained. Subsequent to stripping, the foundation areas should be proof rolled using a heavy wheeled vehicle.

Any “soft/loose” soils noted during the proof rolling or observed within excavations should be removed to a depth where stiffer soils are encountered or to a minimum depth of 2 feet. Excavated soils should be replaced with controlled-compacted structural fill. If fill is needed, the area should be brought to grade using a clean, select, fill material free from debris or organic matter.

A cohesionless soil described as clean sand with less than 10% passing the U.S. No. 200 Sieve may be used for fill. Alternatively, a lean, silty or sandy clay (CL - USCS Classification) may be used for fill. The clay fill should have a Liquid Limit of less than 40 and a Plasticity Index (PI) of less than 20.

6.4 Fill Placement and Compaction

Fill should be placed in 10 to 12-inch loose lifts. Minimum compaction criteria of a dry density at least equal to 95% of its maximum, as determined by the Standard Proctor compaction test (ASTM D698), should be used for fill that will support foundations.

7.0 DEEP FOUNDATIONS FOR HEAVILY LOADED STRUCTURES

As an alternative to shallow foundations, a deep foundation system should be used for the support of structures not feasible for shallow foundations. Consideration should be given to supporting all loads (columns, walls, and floors) on piles if deep foundations are used.

7.1 Allowable Pile Load Capacities

Analyses have been made to determine the estimated allowable pile load capacities for 14-inch square, pre-cast concrete piles and 14-inch diameter open-ended, steel, pipe piles. Allowable pile load capacities are provided on Figures No. 2 and No. 3.

The allowable pile load capacities provide for a 2-foot cutoff below the existing ground surface, assume the piles are vertical, and do not include the weight of the pile. The provided compression capacities contain an estimated factor of safety of 2 against failure of a single pile through the soil. The provided tension capacities contain an estimated factor of safety of 3 against failure. The capacities also include a limiting adhesion value based on load tests in geologically similar soils.

The analyses for pile capacities are based on a soil-pile relationship only. The structural capacity of the piles and their connections to transmit these loads should be determined by a structural engineer.

7.2 Pile Driving

In general, driving of SPC piles should be limited to a rate of 75 blows per foot using a minimum driving energy of 19,500 foot-pounds per blow (Vulcan No. 06 hammer, or equivalent). Driving of OSP piles should be limited to a rate

of 75 blows per foot using a minimum driving energy of 24,000 foot-pounds per blow (Vulcan No. 08 hammer, or equivalent)

Predrilling for pile installation does not appear to be necessary. If used, predrilling should be made with a bit that is no larger than 85% of the pile's tip diameter or side dimensions and should not penetrate to within 5 feet of the pile's design tip depth. Predrilling may also be used to reduce vibrations.

7.3 Probe Piles and Pile Load Tests

It is recommended that probe type piles be installed at the site to establish installation characteristics and pile lengths. The probe piles should be of the same type and size as the job piles and should be installed with the same equipment and techniques that will be used to install the job piles.

We recommend the probe piles be allowed to set for a period of 14 days and at least one of the probe piles be tested to failure in accordance with ASTM D 1143. Gulf South should be retained to evaluate and verify the estimated pile load capacities.

7.4 Vibrations

Vibrations due to construction activities should be expected and they should be monitored during all construction activities. In general, vibrations should be limited to about 0.25 inch/sec. (average peak particle velocity) at all existing nearby sensitive structures. Construction should be stopped if peak values exceed about 0.5 in./sec.

7.5 Drag Load

When fill is placed on the site, the underlying compressible soils consolidate, resulting in surface settlement. As the compressible soils consolidate, "negative skin friction" or downdrag can be imparted on piles. This can result in a load that is additive to structural loads on the piles/shafts and will increase settlement of the piles/shafts and structures.

Drag load is dependent on the thickness of fill, compressibility of the soils, time-rate of consolidation, and pile size and length. Gulf South should be notified if more than 2 feet of fill is expected to be placed on site.

7.6 Group Effect

The effects of pile grouping on single pile load capacities is dependent on pile spacing, pile lengths, and soil characteristics throughout the pile length and below the pile tip. Assuming a minimum center to center spacing of 3 ft., group effect should be unimportant for pile clusters of up to 6 piles. Group effect may become important for larger clusters and should be evaluated when actual pile layouts are known using the criteria provided on Figure No. 3.

7.7 Estimated Settlement for Deep Foundations

Settlement of pile supported footings and slabs constructed in single, widely, spaced rows, or in clusters of up to 4 to 6 piles is estimated to be 1 inch or less for the provided capacities and tip depths. These values assume piles are driven/drilled to the specified tip depths and not loaded greater than the stated allowable carrying capacities.

8.0 CLOSING

Gulf South is available to answer any questions you may have concerning this report. Should additional analyses be required or requested, additional fees may be necessary.

As previously discussed, Gulf South considers the materials testing and onsite inspection during construction an extension of our geotechnical exploration. Gulf South should be retained to provide the construction inspection services.

The issuance of this report completes the geotechnical exploration scope and Gulf South's involvement on the project. Retaining Gulf South as a vital member of the design team can add considerable value. Over the next few months, the project will incur many changes, challenges, and opportunities – all of which will occur without our knowledge and in some cases rendering our recommendations compromised or irrelevant. Gulf South's additional involvement will be a small price to pay for the peace of mind that any foundation, earthwork, and paving components of the project are fully integrated during design, resulting in potential cost savings and efficient


DELTA COAST CONSULTANTS, LLC – HOUMA, LA

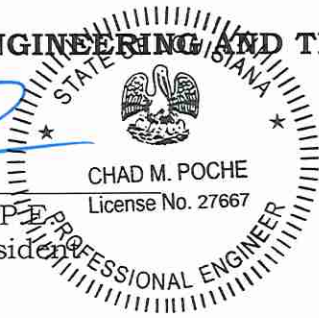
construction. Please consider including Gulf South as a full member of your design team throughout the project duration.

We appreciate the opportunity to provide this report and look forward to working with you again in the future.

Sincerely,

GULF SOUTH ENGINEERING AND TESTING, INC.


CHAD M. POCHE, P.E.
Executive Vice President




BRYSON S. BEARD, E.I.
Associate Geotechnical Engineer

FIGURES

Confidential and Proprietary



N American Ct

Rome Woodard St

B-2

B-1

Rome Woodard St

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GEOTECHNICAL EXPLORATION

PORT OF TERREBONNE
LED SITE CERTIFICATION
ROME WOODWARD DR.
HOUMA, LA

GULF SOUTH ENGINEERING AND TESTING PROJECT NO. 24-004

ALLOWABLE PILE LOAD CAPACITIES

[ASSUMES EFFECTS OF LESS THAN 2 FEET OF FILL]

OPEN-ENDED, STEEL PIPE PILES

PILE SIZE	PILE TIP EMBEDMENT BELOW GROUND SURFACE IN FEET	ESTIMATED ALLOWABLE SINGLE PILE LOAD CAPACITIES IN TONS COMPRESSION FACTOR OF SAFETY = 2 TENSION FACTOR OF SAFETY = 3	
		COMPRESSION	TENSION
14-inch Diameter	30	6	4
	35	7	5
	40	8	5.5
	45	9	6
	50	10	7

GEOTECHNICAL EXPLORATION

PORT OF TERREBONNE
LED SITE CERTIFICATION
ROME WOODWARD DR.
HOUMA, LA

GULF SOUTH ENGINEERING AND TESTING PROJECT NO. 24-004

ALLOWABLE PILE LOAD CAPACITIES

[ASSUMES EFFECTS OF LESS THAN 2 FEET OF FILL]

SQUARE, PRECAST, CONCRETE PILES (SPC)

PILE TYPE AND SIZE (ASTM D25)	PILE TIP EMBEDMENT BELOW EXISTING GROUND SURFACE IN FEET	ESTIMATED ALLOWABLE SINGLE PILE LOAD CAPACITIES IN TONS COMPRESSION FACTOR OF SAFETY = 2 TENSION FACTOR OF SAFETY = 3	
		COMPRESSION	TENSION
14-in x 14-in	30	8	5
	35	9	6
	40	10	7
	45	11	8
	50	13	9

Minimum Pile Spacing

$$SP = 0.05 L_1 + 0.025 L_2 + 0.0125 L_3$$

SP (ft.) = Center to center spacing of piles/shafts = (Min. 3.0 ft.)

L_1 = Pile/Shaft penetration in ft. up to 100 ft.

L_2 = Pile/Shaft penetration in ft. from 101 to 200 ft.

L_3 = Pile/Shaft penetration in ft. from 201 to 300 ft.

Allowable Group Capacity*

$$Q_a = \frac{P * L * c}{FSF} + \frac{2.6 * q_u * (1 + 0.2 w/b) * A}{FSB}$$

P = Average perimeter of pile/shaft group (ft.)

L = Length of piles/shafts in group (ft.)

c = Average (weighted) shear strength ($\frac{1}{2} q_u$) of soil throughout pile/shaft length (lbs./sq. ft.)

q_u = Unconfined compressive strength of soils below pile tips (lbs./sq.ft.)

w = Width of pile/shaft group at tip (ft.)

b = Length of pile/shaft group at tip (ft.)

A = Area of pile/shaft group at tip (sq. ft.)

FSF = Factor of safety for friction area = 2

FSB = Factor of safety for tip area = 3

*In no case should the cumulative single pile/shaft load capacity of the group be exceeded.

APPENDIX

BORING LOGS



15 Veterans Memorial Blvd,
Kenner, LA
Office: +1 (504) 305-4401

**Port of Terrebonne - LED Site Certification - Rome
Woodward Dr.**
Lat/Lon: 29.562842/-90.697111

SOIL BORING: B-1

Date Started: 02/05/24 Date Completed: 02/05/24 Location Accuracy: Tablet GPS
Project No: 24-004 Client Name: Delta Coast Consultants, LLC Boring Diameter: 4"
Driller: J. Anslem Drilling Firm: Gulf South Engineering and Testing Hammer Type: Cathead
Hammer Weight: 140 lbs Logged By: K. Daigle Method: Mud Rotary
Depth: 50'

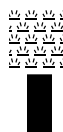
Depth (ft)	Sample Graphic	Lab						Graphic Log	Rig Type Tooling Surface Elevation	Ardco K-1000 Rotary Drill ~N/A
		Blow Counts (N-Value)	Compressive Strength (tsf)	Moisture Content (%)	Wet Density (PCF)	Atterberg Limits (LL-PL-Pi)	% Fines		Visual Classification and Remarks	
5	1.50		0.514	37.4	86	101-39-62			Medium Stiff, gray and brown, FAT CLAY (CH) , with organics trace gravel	2.0
	0.50		0.17	118.2	87	157-40-117			Very Soft, dark gray, ORGANIC CLAY (OH) , with wood	
	0.25		0.158	109.7	79	194-51-143				8.0
	0.00		0.109	152.5	80				Medium Stiff, dark gray, PEAT	10.0
10	0.00		0.77	258.4	75				Very Soft, dark gray, ORGANIC CLAY (OH) , with wood	
	0.25			116.5		194-77-117				18.0
15	0.00		0.083	105.7	80				Very Soft, dark gray, PEAT	23.0
20	0.00			254.2					Very Soft, dark gray, PEAT	
25	0.00		0.105	91.7	89				Very Soft to Soft, gray and light gray, FAT CLAY (CH) , with organics	
30	0.00			109.8						
35	0.25		0.172	64.9	99					

Graphics Legend



CH

OH



Peat

ST - Shelby Tube

REMARKS

- Borehole backfilled per LA DOTD & LA DEQ requirements upon completion.







15 Veterans Memorial Blvd,
Kenner, LA
Office: +1 (504) 305-4401

**Port of Terrebonne - LED Site Certification - Rome
Woodward Dr.**
Lat/Lon: 29.562842/-90.697111

SOIL BORING: B-1

Date Started: 02/05/24 Date Completed: 02/05/24 Location Accuracy: Tablet GPS
Project No: 24-004 Client Name: Delta Coast Consultants, LLC Boring Diameter: 4"
Driller: J. Anslem Drilling Firm: Gulf South Engineering and Testing Hammer Type: Cathead
Hammer Weight: 140 lbs Logged By: K. Daigle Method: Mud Rotary
Depth: 50'

Depth (ft)	Sample Graphic	Lab						Graphic Log	Rig Type Tooling Surface Elevation	Ardco K-1000 Rotary Drill ~N/A
		Pocket Penetrometer (tsf)	Blow Counts (N-Value)	Compressive Strength (tsf)	Moisture Content (%)	Wet Density (PCF)	Atterberg Limits (LL-PL-P)			
									Visual Classification and Remarks	
40		0.00		0.194	109.5	88			Very Soft to Soft, gray and light gray, FAT CLAY (CH) , with organics	
45		0.00		0.2	88.7	93				
50		0.00		0.283	57.1	103				
Soil boring completed at 50 ft. below the ground surface.										
										50.0

Graphics Legend



CH



ST - Shelby Tube

REMARKS

- Borehole backfilled per LA DOTD & LA DEQ requirements upon completion.



15 Veterans Memorial Blvd,
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**Port of Terrebonne - LED Site Certification - Rome
Woodward Dr.**
Lat/Lon: 29.565042/-90.697119

SOIL BORING: B-2

Date Started: 02/05/24 Date Completed: 02/05/24 Location Accuracy: Tablet GPS
Project No: 24-004 Client Name: Delta Coast Consultants, LLC Boring Diameter: 4"
Driller: J. Anslem Drilling Firm: Gulf South Engineering and Testing Hammer Type: Cathead
Hammer Weight: 140 lbs Logged By: K. Daigle Method: Mud Rotary
Depth: 30'

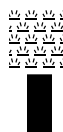
Depth (ft)	Sample Graphic	Lab						Graphic Log	Rig Type Tooling Surface Elevation	Ardco K-1000 Rotary Drill ~N/A
		Blow Counts (N-Value)	Compressive Strength (tsf)	Moisture Content (%)	Wet Density (PCF)	Atterberg Limits (LL-PL-P)	% Fines		Visual Classification and Remarks	
		0.50	0.524	65.4	96	108-46-62			Medium Stiff, dark gray and brown, FAT CLAY (CH) , with organics	2.0
		0.25	0.129	151.3	87				Very Soft, dark gray, ORGANIC CLAY (OH) , with wood	4.0
5		0.25	0.084	251.9	81	292-92-200			Very Soft, dark gray and black, PEAT , with trace organics	6.0
		0.00	0.116	132.2	84	183-46-137			Very Soft, dark gray, ORGANIC CLAY (OH) , with organics	8.0
10		0.00		303.8					Very Soft, dark gray and black, PEAT , with organics	
		0.00	0.04	236.6	77					
		0.00	0.063	253.6	73					
15										
		0.25		75.8					Very Soft, gray, FAT CLAY (CH) , with organics and Wood	18.0
20										
		0.00	0.131	72.5	100					
25										
		0.25	0.232	60.1	103		99			30.0
30									Boring completed 30 ft. below the ground surface.	

Graphics Legend



CH

OH




Peat



ST - Shelby Tube

REMARKS

- Borehole backfilled per LA DOTD & LA DEQ requirements upon completion.



GULF SOUTH

ENGINEERING AND TESTING, INC.
Geotechnical & Materials Consultants

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